Appendix L

Multimodal Transportation Impact Analysis Report



San Luis Ranch Specific Plan

Multimodal Transportation Impact Analysis Report

Prepared for:

City of San Luis Obispo

Prepared by:



SAN LUIS RANCH SPECIFIC PLAN MULTIMODAL TRANSPORTATION IMPACT ANALYSIS REPORT

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- G. Sidra Outputs
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Executive Summary

This study has been prepared by Omni-Means to evaluate the potential multimodal transportation impacts associated with development of the San Luis Ranch Specific Plan (Project) located in San Luis Obispo County. The Project is located on a 131.3-acre site (previously known as Dalidio Ranch) in the City of San Luis Obispo's Sphere of Influence. The Project site would be annexed by the City upon approval of the Specific Plan. The site is located between US 101 and Madonna Road, located roughly midway between the existing US 101 interchanges at Los Osos Valley Road and Madonna Road. The mixed-use Project proposes to develop 580 single-family and multifamily residential units, a 200-room hotel, and 250,000 square feet of commercial (retail and office) space.

In keeping with City policies and requirements for transportation impact analysis, this study quantifies impacts to all modes of travel (auto, bike, pedestrian, and transit), and estimates project generation of new person trips using all modes of travel available. Due to the large size of the Project and the mix of proposed on-site land uses, the Project's trip generation estimates also consider internally capture trips between different uses, such as residential to commercial, and commercial to commercial. Table ES-1 presents a summary of the Project's trip generation, by mode.

TABLE ES-1: CUMULATIVE PROJECT TRIP GENERATION SUMMARY

Category	AM Peak Hour Trips	Share	PM Peak Hour Trips	Share
Internal Person Trips	68	6.4%	382	19.8%
External Person Trips	1,009	93.6%	1,544	80.2%
Total Person Trips	1,077	100%	1,926	100%
External Trips by Mode	AM Peak Hour Trips	Share	PM Peak Hour Trips	Share
Automobile (Total Persons)	914	90.5%	1,435	93%
Automobile (Total Cars)	648	N/A	882	N/A
Bicycle	34	3.4%	32	2.1%
Pedestrian	50	4.9%	56	3.6%
Transit	12	1.2%	20	1.3%
Total External Trips	1,009	100%	1,544	100%

Multimodal Analysis and Performance Metrics

For automobile travel mode, intersection capacity analysis was performed using HCM 2010 methodologies in Synchro. Performance is measured in delay-based level-of-service (LOS), and impact significance is measured by either the creation of a new deficiency, or the worsening of a pre-project deficient condition by a threshold change in approach or movement volume-to-capacity. This means that an intersection that has been determined to operate unacceptably without the project will only be considered a project-specific impact if the project adds a certain level of traffic to that location. Bicycle and pedestrian intersection LOS is based on a "score" determined through HCM 2010 methods, implemented through Synchro. As with auto mode, a threshold change in score was utilized to determine impact significance at intersections where pedestrian or bicycle LOS is identified as deficient in the pre-project condition.

Queuing analysis was performed using SimTraffic, and impact significance was measured by either the creation of a new queuing deficiency, or the exacerbation of an existing queuing deficiency. Absent a threshold metric to determine project impact at locations where queues exceed storage capacity in the pre-project condition, any queuing issue that is identified in the pre-project condition is considered a significant project impact regardless of how much the queue capacity impact is increased with the addition of project traffic.

Roadway segment analysis was performed using HCM 2010 methodologies in a Multimodal LOS (MMLOS) spreadsheet developed by Omni-Means. For auto mode, the performance metric is LOS based on segment travel speed. Impact significance was measured by either the creation of a new roadway segment deficiency or the worsening of a pre-project deficient segment by a threshold decrease in travel speed. This means that a roadway segment that has been determined to operate unacceptably without the project will only be considered a project-specific impact if the project reduces travel speed by a certain level along that location. For pedestrian, bicycle, and transit modes, LOS is based on a "score" determined through HCM 2010 methods. As with roadway segment, impact significance was measured by either the creation of a new deficiency or by a threshold degradation of segment score where a pre-project deficiency is identified.

Bicycle, Pedestrian, and Transit "Impact" Findings

Based on current California Environmental Quality Act (CEQA) guidelines, automotive LOS is currently the travel mode being used as the basis for CEQA impact and significance findings. CEQA is also currently undergoing changes with the adoption of Senate Bill 743, which replaces automotive level of service with Vehicle Miles Traveled as the environmental impact metric. Unlike the automotive mode, the bicycle, pedestrian, and transit impact analysis is performed at the discretion of the City of San Luis Obispo on an informational basis to ensure broad disclosure of possible existing and future pre-project conditions and project impacts. The current HCM 2010 methodology for non-motorized impacts has limited contextual application on highervolume roadway segments, for example HCM does not consider various potential mitigation measures such as class IV bike lanes or parallel class I facilities. High automotive traffic volumes make it difficult to mitigate pedestrian and bicycle impacts without significant changes to the built environment that go well beyond the provision of appropriate crosswalks and sidewalks. Until HCM methodologies are modified to account for these shortfalls, the City's transportation impact study guidelines provide for consideration of mitigation measures not yet fully incorporated into the HCM methodology to address mitigating pedestrian and bicycle LOS impacts.

Prado Road Overcrossing Only Vs. Full Access Interchange Findings

As part of this traffic impact study a complete and thorough assessment of the Prado Rd. Interchange and its timing was completed. The study concluded that an Overcrossing with only NB ramps would provide for adequate operations under existing and near term conditions; however a full access interchange with both NB and SB Ramps could be differed until buildout conditions. The Overcrossing with only NB ramps would be needed as part of Phase 2 of the San Luis Ranch development.

Impacts & Mitigation Summary

The San Luis Ranch Project was evaluated under three scenarios; existing + project conditions, near term + project conditions, and cumulative + project conditions. Existing + project conditions reflect the impacts if the entire project were constructed today. Because the project is proposed to be built out over a 5-10 year period the project was also evaluated under near term + project conditions to also reflect other growth and infrastructure improvements anticipated to occur during that time. The combination of analysis of existing and near term conditions provides the basis for establishing phase triggers for project mitigation requirements and project proposed infrastructure. Cumulative + project conditions reflect the impacts of the project under the planned buildout of the City.

TABLE ES-2 EXISTING & NEAR-TERM IMPACTS & MITIGATIONS

EXISTING & NEAR TERM + PROJECT IMPACTS & MITIGATION MEASURES				
	Location	Impact	Mitigation	Trigger
Int. #1	Madonna & LOVR	Volume Exceeds Lane Capacity	Construct Prado Overpass	Phase 2
		Unacceptable Level of Service		
Int. #2	Madonna & Oceanaire	Volume Exceeds Lane Capacity	Construct Prado Overpass	Phase 2
Int. #3	Madonna & Dalidio/Prado	Unacceptable Level of Service	a. Extend Madonna WB Left to 310'	Phase 1
		Volume Exceeds Lane Capacity	b. Install 2nd 310' Madonna WB Left	Phase 1
			c. Install Dedicated EB Madonna Right	Phase 1
			d. Prohibit WB Madonna U-Turns	Phase 1
			e. Reconfigure and Optimize Signal Timing with	
			Split Phasing	Phase 1
			f. Install 2nd Prado/Dalidio NB Left	With Prado Construction
Int. #4	Madonna & El Mercado	Volume Exceeds Lane Capacity		Phase 1
Int. #5	Madonna & US 101 SB Ramp	Unacceptable Level of Service	Construct Prado Overpass	Phase 2
			Extend EB Madonna Left to Madonna Inn to 150'	Phase 1
Int. #6	Madonna & US 101 NB Ramp	Volume Exceeds Lane Capacity	Construct Prado Overpass	Phase 2
Int. #7	Madonna & Higuera		Construct Prado Overpass & NB Ramps	Phase 2
Int. #9	LOVR & Froom	Volume Exceeds Lane Capacity	Install Dedicated EB Froom Right Turn 230'	With Froom Bridge Construction
			Extend WB Froom Right Turn to 110'	With Froom Bridge Construction
			Construct 2nd WB Froom Left turn Lane	With Froom Bridge Construction
Int. #10	LOVR & Autopark	Unacceptable Level of Service	Signalize Intersection	Phase 1
			Construct Prado Overpass	Phase 2
Int. #11	LOVR & Calle Joaquin	Volume Exceeds Lane Capacity	Construct Prado Overpass	Phase 2
Int. #12	LOVR & SB 101 Ramps		Extend Off-Ramp Left Turn Pocket to 320'	Phase 1
Int. #13	LOVR & NB 101 Ramps	Volume Exceeds Lane Capacity		Phase 2
Int. #14	LOVR & Higuera		Extend EB LOVR Right Turn Pocket to 180'	Phase 1
Int. #15	Higuera & Suburban	Volume Exceeds Lane Capacity		Phase 2
Int. #16	Higuera & Tank Farm	Unacceptable Level of Service	Construct Prado Overpass	Phase 2
		Volume Exceeds Lane Capacity	Extend NB Right and Install Channelized / Yield	Phase 1
			Control	
Int. #18	Prado & Higuera	Volume Exceeds Lane Capacity	Construct 2nd Higuera NB Left	Phase 1
			Extend WB Prado Right to 410'	Phase 1
Int. #21	Prado/Dalidio & Driveway	Unacceptable Level of Service	Construct Multilane Roundabout	With Intersection Construction
Int. #25	Prado/Dalidio & Froom	Unacceptable Level of Service	Construct Multilane Roundabout	With Intersection Construction
Seg. #1	Madonna (LOVR to Higuera)	Unacceptable Level of Service	Construct Prado Overpass	Phase 2
Seg. #7	Higuera (Madonna to Prado)	Unacceptable Level of Service	Construct Parallel Class I Path	With Prado Construction
Seg. #13	LOVR (Madonna to Froom)	Unacceptable Level of Service	Construct Prado Overpass	Phase 2
Seg. #17	LOVR (NB Ramps to Higuera	Unacceptable Level of Service	Construct Prado Overpass & NB Ramps	Phase 2
	Prado/Dalidio (Froom to	Unacceptable Level of Service	Construct Parallel Class I Path	With Prado Construction
Seg. #18	Higuera)			
	Prado/Dalidio (Froom to	Unacceptable Level of Service	Construct Parallel Class I Path	With Prado Construction
Seg. #20	Madonna)			

TABLE ES-3 CUMULATIVE IMPACTS & MITIGATIONS

CUMULATIVE + PROJECT IMPACTS & MITIGATION MEASURES					
	Location	Impact	Mitigation		
Int. #1	Madonna & LOVR	Volume Exceeds Lane Capacity	Existing & Near Term Mitigation		
			Extend NB LOVR Right to 295'		
			Extend SB Madonna Left to 395'		
Int. #2	Madonna & Oceanaire	Volume Exceeds Lane Capacity	Existing & Near Term Mitigation		
			Extend WB Madonna Right to 200'		
Int. #3	Madonna & Prado/Dalidio	Unacceptable Level of Service	Existing & Near Term Mitigation		
		Volume Exceeds Lane Capacity	Extend EB Madonna Left to 145'		
Int. #4	Madonna & El Mercado	Volume Exceeds Lane Capacity	Existing & Near Term Mitigation		
Int. #5	Madonna & 101 SB	Volume Exceeds Lane Capacity	Existing & Near Term Mitigation		
			Construct NB & SB Ramps		
Int. #6	Madonna & 101 NB	Volume Exceeds Lane Capacity	Existing & Near Term Mitigation		
			Construct NB & SB Ramps		
Int. #8	Higuera & South	Volume Exceeds Lane Capacity	Extend NB Higuera Left Turn Pocket to 120'		
	_		Extend EB South St Right Turn Pocket to 100'		
Int. #9	LOVR & Froom	Volume Exceeds Lane Capacity	Existing & Near Term Mitigation		
Int. #10	LOVR & Auto Park	Unacceptable Level of Service	Existing & Near Term Mitigation		
Int. #11	LOVR & Calle Joaquin	Volume Exceeds Lane Capacity	Construct Prado NB & SB Ramps		
Int. #12	LOVR & 101 SB	Unacceptable Level of Service	Existing & Near Term Mitigation		
		Volume Exceeds Lane Capacity	Construct Prado Overpass with NB & SB Ramps		
Int. #13	LOVR & 101 NB	Unacceptable Level of Service	Existing & Near Term Mitigation		
		Volume Exceeds Lane Capacity	Construct NB & SB Ramps		
Int. #14	LOVR & Higuera	Unacceptable Level of Service	Existing & Near Term Mitigation		
		Volume Exceeds Lane Capacity	Construct Prado Overpass with NB & SB Ramps		
Int. #16	Higuera & Tank Farm	Volume Exceeds Lane Capacity	Existing & Near Term Mitigation		
			Construct NB & SB Ramps		
Int. #18	Higuera & Tank Farm	Volume Exceeds Lane Capacity	Existing & Near Term Mitigation		
Seg. #1	Madonna (Higuera - LOVR)	Unacceptable Level of Service	Existing & Near Term Mitigation		
			Construct NB & SB Ramps		
Seg. #5-#6	Madonna (SB Ramps - Higuera)	Unacceptable Level of Service	Construct Prado Overpass with NB & SB Ramps		
Seg. #7	Higuera (Madonna to Prado)	Unacceptable Level of Service	Existing & Near Term Mitigation		
Seg. #15-#16	LOVR (Calle Joaquin - 101 NB)	Unacceptable Level of Service	Construct Prado Overpass with NB & SB Ramps		
Seg. #24	Prado/Dalidio (Driveway - Froom)	Unacceptable Level of Service	Construct Prado Overpass with NB & SB Ramps		

Introduction

The City of San Luis Obispo has retained Omni-Means to perform a Multimodal Transportation Impact Study (TIS) for the proposed San Luis Ranch Specific Plan. The proposed mixed-use development is located on a 131.3-acre site in unincorporated San Luis Obispo County, adjacent to the City of San Luis Obispo, and within the City's Sphere of Influence (SOI). The site is generally bounded by Madonna Road, Dalidio Drive, and US Highway 101. The site is part of an agricultural reserve that has historically been used as farmland. Consistent with the requirements of the General Plan, the San Luis Ranch Specific Plan must be adopted by the City Council prior to annexation of the Plan Area. The City would annex the Plan Area with project approval. The San Luis Ranch Specific Plan is proposing a mix of residential, commercial, hotel, and office uses while preserving substantial areas of open and agricultural space. This multimodal transportation impact study would evaluate the proposed 580-Unit Alternative of the San Luis Ranch Specific Plan to determine any operational or safety impacts to the surrounding infrastructure. This Multimodal TIS presents the projected transportation operations and impacts associated with development of the project under Existing, Near Term, and Cumulative Conditions for vehicular, pedestrian, bicycle, and transit related impacts, and the mitigation measures required to mitigate impacts to less than significant.

Intersections

The City of San Luis Obispo has identified and pre-selected twenty (20) existing intersections for analysis. In addition, intersections 21-28 are project-only intersections that would be constructed in conjunction with the proposed project and which would be evaluated either as part of the plus project conditions only.

- 1. Madonna Road / Los Osos Valley Road
- 2. Madonna Road / Oceanaire Drive
- 3. Madonna Road / Dalidio Drive
- 4. Madonna Road / El Mercado
- 5. Madonna Road / US 101 Southbound Ramps
- 6. Madonna Road / US 101 Northbound Ramps
- 7. Madonna Road / Higuera Street
- 8. Higuera Street / South Street
- 9. Los Osos Valley Road / Froom Ranch Way
- 10. Los Osos Valley Road / Auto Park Way
- 11. Los Osos Valley Road / Calle Joaquin
- 12. Los Osos Valley Road / US 101 Southbound Ramps
- 13. Los Osos Valley Road / US 101 Northbound Ramps
- 14. Los Osos Valley Road / S. Higuera Street
- 15. S. Higuera Street / Suburban Drive
- 16. S. Higuera Street / Tank Farm Road
- 17. S. Higuera Street / Granada Drive
- 18. S. Higuera Street / Prado Road
- 19. S. Higuera Street / Margarita Avenue
- 20. Prado Road / US 101 Northbound Ramps/Elks Lane

Future Intersections

- 21. Froom Ranch Way / Dalidio Drive (Future)
- 22. Madonna Road / Project Driveway #1 (*Project Internal*)

- 23. Froom Ranch Way / Project Driveway #2 (Project Internal)
- 24. Dalidio Drive / US 101 SB Ramps (Future)
- 25. Dalidio Drive / Project Shopping Center Driveway (*Project Internal*)
- 26. Dalidio Drive / Post Office Driveway / Promenade (Access Management)
- 27. Froom Ranch Way / Hotel Project Driveway (*Project Internal*)
- 28. Froom Ranch Way / Project Driveway #3 (Project Internal)

Roadway Segments

Ten (10) arterial roadways have been identified and pre-selected by the City of San Luis Obispo for analysis. For the purpose of this analysis, the segments have been broken up into 24 separate segments in between major intersections, as follows:

Madonna Road:

- 1. Los Osos Valley Road to Oceanaire Drive
- 2. Oceanaire Drive to Dalidio Drive
- 3. Dalidio Drive to El Mercado
- 4. El Mercado to US 101 Southbound Ramps
- 5. US 101 Southbound Ramps to US 101 Northbound Ramps
- 6. US 101 Northbound Ramps to Higuera Street

S. Higuera Street:

- 7. Madonna Road to Margarita Avenue
- 8. Margarita Avenue to Prado Road
- 9. Prado Road to Granada Drive
- 10. Granada Drive to Tank Farm Road
- 11. Tank Farm Road to Suburban Road
- 12. Suburban Road to Los Osos Valley Road

Los Osos Valley Road:

- 13. Madonna Road to Froom Ranch Way
- 14. Froom Ranch Way to Calle Joaquin
- 15. Calle Joaquin to US 101 Southbound Ramps
- 16. US 101 Southbound Ramps to US 101 Northbound Ramps
- 17. US 101 Northbound Ramps to S. Higuera Street

Prado Road

18. US 101 Northbound Ramps to Higuera Street

Froom Ranch Way

- 19. Los Osos Valley Road to Dick's Sporting Goods Driveway
- 21. Dick's Sporting Goods Driveway to Dalidio Drive (*Future*)

Dalidio Drive

- 20. Madonna Road to Froom Ranch Way (*Future*)
- 22. US 101 Northbound Ramps to US 101 Southbound Ramps (*Future*)

- 23. US 101 Southbound Ramps to Froom Ranch Way (Future)
- 24. Froom Ranch Way to Shopping Center Driveway (*Future*)

The *Future* roadway segments, which are either internal to the project or new roadways depending on the analysis scenario, are evaluated in between traffic signals or major cross-section changes.

Highway Segments

Two highway mainline segments have been identified and pre-selected by the City of San Luis Obispo and Caltrans for analysis. Average Daily Traffic (ADT) counts were obtained from Caltrans for the two following segments:

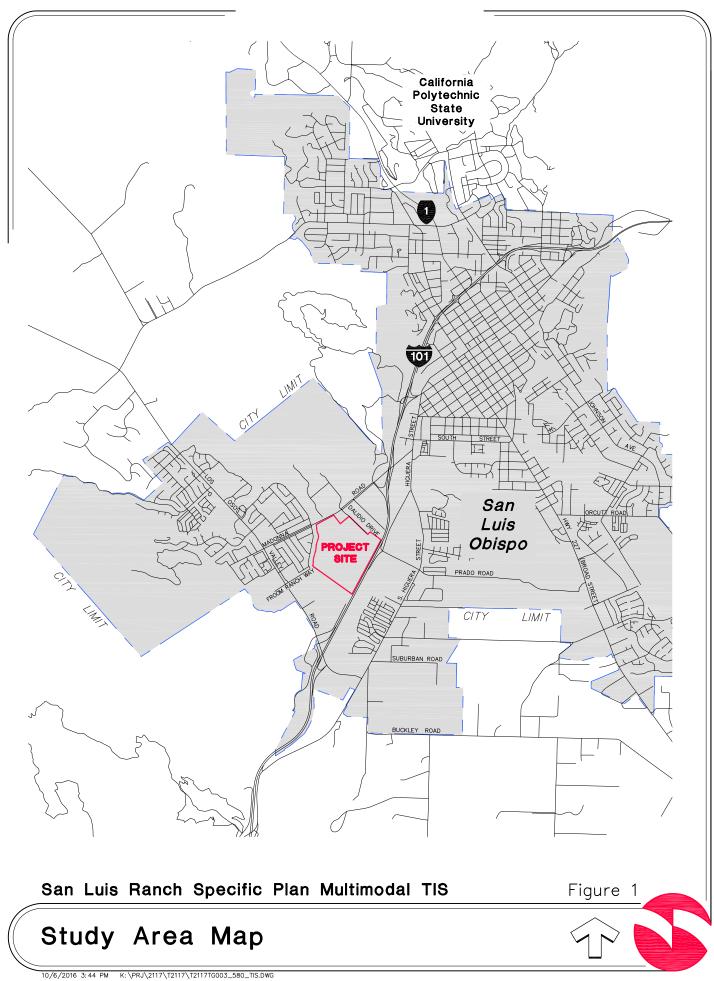
- 1. US 101 between Los Osos Valley Road and Prado Road
- 2. US 101 between Prado Road and Madonna Road

The study locations above were analyzed for weekday AM and PM peak hours for both intersections and roadway segments using vehicular, bicycle, and pedestrian counts collected either from the City's on-line traffic counts database or by Omni-Means. The following traffic scenarios were analyzed as part of this study:

- Existing Conditions
- Existing Plus Project Conditions
- Near Term (Year 2025) Conditions
- Near Term Plus Project Conditions
- Cumulative (Year 2035) Full Build Prado Road Interchange Conditions
- Cumulative Full Build Prado Road Interchange Plus Project Conditions
- Cumulative (Year 2035) Prado Road Overcrossing Conditions
- Cumulative Prado Road Overcrossing Plus Project Conditions

Existing conditions establishes the baseline conditions for the year 2014 traffic operations, when the traffic counts were collected, at the study locations. Existing Plus Project conditions is an analysis scenario in which project-related traffic impacts are examined in comparison to the Existing conditions. The Near Term conditions is an analysis scenario in which the City's approved, pending and potential land development projects are assumed to be in place, in roughly 10 years (Year 2025). The Near Term Plus Project conditions is an analysis scenario in which project-related traffic impacts are examined in comparison to the Near Term conditions.

Cumulative conditions establish the conditions that would exist at build-out of the City's General Plan, representing approximately twenty years out (Year 2035). Cumulative Plus Project conditions is an analysis scenario in which project-related traffic impacts are examined in comparison to Cumulative conditions. Two cumulative scenarios have been included in this study. The Cumulative Full Build Prado Road Interchange analysis scenarios consider a full access diamond interchange to be constructed at Prado Road, in addition to all other City roadway improvements. The Cumulative Year 2035 Prado Road Overcrossing scenarios consider an overcrossing to be constructed over US 101 connecting Prado Road to Dalidio Drive and the existing US 101 Northbound ramps to be removed, in addition to all other City roadway improvements in place. In both cumulative scenarios, improvements to the City's transportation infrastructure system are assumed to be constructed, consistent with the General Plan Circulation Element. Figure 1 presents the study area and vicinity map.



Multimodal Analysis Methodology and Technical Parameters

The following section outlines the analysis methodology and technical parameters used to quantify operations for all transportation modes in the TIS.

Applicable Multimodal Level of Service Policies

City of San Luis Obispo Policy

The City of San Luis Obispo *Circulation Element* contains Level of Service (LOS) policies for all modes of transportation. The City's goal is to maintain the LOS objective. However if the project causes the LOS to exceed the minimum LOS standard, the project is considered to have caused an impact.

Table 1 shows the standard acceptable LOS threshold by mode.

TABLE 1:
LOS OBJECTIVE AND MINIMUM STANDARD FOR ALL MODES OF TRANSPORTATION

Travel Mode	Objective LOS	Minimum LOS Standard
Bicycle	В	D
Pedestrian	В	С
Transit	С	Baseline LOS or LOS D, whichever is lower
Vehicle	С	E (Downtown), D (All Other Routes)

Consistent with City policies, the LOS threshold for bicyclist has been LOS "D", pedestrian LOS threshold has been LOS "C", transit LOS threshold has been the lower of baseline LOS or LOS "D", and vehicle LOS threshold has been "D". The objective LOS for bicycle and pedestrian modes is LOS "B", and for transit and vehicle modes is LOS "C".

Caltrans Policy

In addition to the City's policies, Caltrans has also established the measure of effectiveness (MOE) for the evaluation of impacts in CEQA level projects on State facilities. Caltrans' Guide for the Preparation of Traffic Impact Studies (December 2002) contains the following policy pertaining to the LOS standards within Caltrans jurisdiction:

The Level of Service (LOS) for operating State highway facilities is based upon measures of effectiveness (MOEs). These MOEs describe the measures best suited for analyzing State highway facilities (i.e., freeway segments, signalized intersections, on- or off-ramps, etc.) Caltrans endeavors to maintain a target LOS at the transition between LOS "C" and LOS "D" on State highway facilities, however, Caltrans acknowledges that this may not always be feasible and recommends that the lead agency consult with Caltrans to determine the appropriate target LOS. If an existing State highway facility is operating at less than the appropriate target LOS, the existing MOE should be maintained.

Consistent with Caltrans policy, the study considers LOS D as the standard acceptable threshold for State highway facilities, such as US 101 and ramp terminals.

Significance Thresholds by Mode

Caltrans Significance Threshold

Based on standard industry practice, the project is considered to have a significant impact if it would:

- Result in a facility that will operate at an acceptable LOS in the No Project condition to deteriorate to an unacceptable LOS in the Plus Project condition; or,
- Increase the density by more than 5% at a facility that will operate at an unacceptable LOS in the

Consistent with the City's adopted Multimodal Transportation Impact Study Guidelines, the project impacts are considered significant if:

Automobiles: Intersections

- A. <u>Signalized Intersections:</u> Project traffic causes minimum LOS standards to be exceeded or further degrades already exceeded LOS standards and the V/C ratio is increased by 0.01 or more.
- B. <u>Unsignalized Intersections:</u> Project traffic causes minimum LOS standards to be exceeded or further degrades already exceeded LOS standards, the V/C ratio is increased by 0.01 or more, and a traffic signal warrant analysis is satisfied.
- C. Project traffic causes or exacerbates 95th percentile turning movement queues exceeding available turn pocket capacity.
- D. The project proposes roadway geometry changes that cause minimum LOS standards to be exceeded or further degrades already exceeded LOS standards for the overall intersection or individual lane groups.

Automobiles: Segments

- A. Project traffic causes minimum LOS standards for either direction to be exceeded or further degrades already exceeded LOS standards and the average segment speed decreases by 1 mph or more.
- B. The project proposes roadway geometry changes that cause minimum LOS standards to be exceeded or further degrades already exceeded LOS standards.

Pedestrian, Bike, & Transit: Intersections & Segments

- A. Project traffic causes minimum LOS standards to be exceeded or further degrades already exceeded LOS standards and there is contextual significance to the impact.
- B. <u>Pedestrians & Bicycles:</u> The project proposes roadway geometry changes that cause minimum LOS standards to be exceeded or further degrades already exceeded LOS standards.

Modal Priorities

In addition to maintaining minimum LOS, the City's Circulation Element has established priorities for various modes such that construction, expansion, or alteration of one mode should not degrade the LOS of a higher priority mode. Project impacts are considered significant if the project proposes to improve a lower priority mode resulting in the degradation of a higher priority mode. Also, if a project's mitigation would result in the degradation of higher priority mode that shall be considered a residual impact and addressed as well. The City's established different Modal LOS Priorities by area are presented in Table 2.

TABLE 2: MODAL PRIORITY RANKING

Complete Street Areas	Priority Mode Ranking		
Dougloup & Upper Montaray Street	1. Pedestrians	3. Transit	
Downtown & Upper Monterey Street	2. Bicycles	4. Vehicle	
Residential Corridors & Neighborhoods	1. Pedestrians	3. Vehicle	
	2. Bicycles	4. Transit	
Commercial Corridors & Areas	1. Vehicles	3. Transit	
Commercial Comdors & Aleas	2. Bicycles	4. Pedestrians	
Regional Arterial and Highway Corridors	1. Vehicles	3. Bicycles	
Tregional Artenal and Highway Comdons	2. Transit	4. Pedestrians	

Based on Table 2, the proposed project would fall under the Regional Arterial and Highway Corridors Priority Mode Ranking for multi-modal level of service (MMLOS), meaning vehicular impacts will carry the most significance. This ranking prioritizes the modes of transportation in the following order: vehicles, transit, bicycles, and pedestrians.

Traffic Safety

A safety assessment is conducted for the study intersections and segments based on the project's potential operational and geometric affects, including turn pocket queue spillbacks and a functional area analysis of project driveways in close proximity to other intersections on collector and arterial roadways.

- A. Project traffic affects the operational characteristics of an existing intersection or segment such that the collision rate, per million entering vehicles for intersections and per million vehicle miles for segments, would be likely to increase.
- B. Project proposed significant new or modified infrastructure is predicted to have a collision rate as indicated above higher than the median collision rate as reported in the City's most current annual traffic survey report for the same facility classification type.

Neighborhood Traffic Analysis

Since this project includes new local residential streets, an evaluation of neighborhood traffic conditions is included. Project impacts are considered significant if the maximum neighborhood ADT or speed thresholds established from the Circulation Element are exceeded or the project adds traffic to a neighborhood already exceeds the ADT threshold. The ADT and speed thresholds established in the Circulation Element are as follows:

- Local Residential maximum ADT is 1,500 vpd and maximum speed is 25 mph;
- Residential Collector maximum ADT is 3,000 vpd and maximum speed is 25 mph.

SB 743: Vehicle Miles Traveled Analysis

CEQA is currently undergoing significant changes with respect to how transportation impacts are considered as an environmental issue. Under SB 743 the California Office of Planning and Research is developing VMT or vehicle miles travelled as the new criteria to replace auto level of service as an environmental impact metric under CEQA. Official thresholds have not yet been adopted; therefore this analysis reports VMT for the project but does not identify impacts or mitigations associated with VMT in the absence of adopted measures.

Mitigation Measures

When significant impacts are identified as part of the traffic impact analysis mitigation measures shall be included to address those impacts. The impact study should establish the legal nexus between the project and the mitigation measures. The traffic study's description of each mitigation measure should include the following:

- 1. Comparison Table of impacted locations listing conditions (i.e. LOS) with and without mitigation.
- 2. Figure schematically depicting location and nature of each mitigation measure and description of implementation feasibility (i.e. ROW requirements, constructability, etc.)
- 3. If specifically scoped planning level cost estimation of each mitigation measure, timing/phasing of measures, and equitable share calculation.

Intersection Control Type Selection

The City's General Plan Circulation Element policies for traffic management on the *Street Network* (Policy 7.1.2) and *Streetscapes and Major Roadways* (Policy 9.2.6) establish that where feasible, roundabouts shall be the City's preferred intersection control alternative due to the vehicle speed reduction, safety, improved aesthetics, reduction in impervious surface areas, additional landscaping areas, and operational benefits of roundabouts. Per these policies, roundabout control should be the first and preferred mitigation measure considered when there are not unworkable right-of-way issues, environmental factors, or other design constraints. When roundabout control is infeasible, consideration and evaluation of multi-way stop control or signalized control should utilize the California MUTCD's appropriate warrants.

Multimodal Level of Service Methodologies

Traffic operations were be quantified through the determination of "Level of Service" (LOS). Level of Service is a qualitative measure of traffic operating conditions, whereby a letter grade "A" through "F" is assigned to an intersection, or roadway segment, representing progressively worsening traffic conditions. Levels of Service were calculated for all intersection control types, urban street segments, and freeway segments using the methods documented in the Transportation Research Board Publication *Highway Capacity Manual, Fifth Edition, 2010* (HCM 2010). The HCM methodologies for intersections were implemented using Synchro (version 9). The Synchro Outputs can be found in the Appendix . SimTraffic software was used to report the 95th percentile queues and are included in the Appendix. Roundabout analysis is conducted using SIDRA 6 software, if applicable. The urban street segment analysis is conducted manually using a spreadsheet which Omni-means has created to formulate the equations set forth in

Chapter 17: *Urban Street Segments* of the HCM 2010, and the inputs and outputs of the spreadsheet can be found in the Appendix. The HCM methodologies for basic freeway segments and ramp junctions were implemented using Highway Capacity Software (*HCS 2010, McTrans*), and is included in the Appendix. The vehicular delay-based LOS criteria for different types of intersection control are outlined in Table 3.

TABLE 3: INTERSECTION LOS CRITERIA FOR VEHICLES

				Stopped Delay	/Vehicle (sec)
Level of Service		Delay	Maneuverability	Signalized/ Roundabouts	Unsignalized/ All-Way Stop
A	Stable Flow	Very slight delay. Progression is very favorable, with most vehicles arriving during the green phase not stopping at all.	Turning movements are easily made, and nearly all drivers find freedom of operation.	< 10.0	< 10.0
В	Stable Flow	Good progression and/or short cycle lengths. More vehicles stop than for LOS A, causing higher levels of average delay.	Vehicle platoons are formed. Many drivers begin to feel somewhat restricted within groups of vehicles.	>10.0 and < 20.0	>10.0 and < 15.0
С	Stable Flow	Higher delays resulting from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant, although many still pass through the intersection without stopping.	Back-ups may develop behind turning vehicles. Most drivers feel somewhat restricted.	>20.0 and < 35.0	>15.0 and < 25.0
D	Approaching Unstable Flow	The influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high volume-to-capacity ratios. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.	Maneuverability is severely limited during short periods due to temporary back-ups.	>35.0 and < 55.0	>25.0 and < 35.0
E	Unstable Flow	Generally considered to be the limit of acceptable delay. Indicative of poor progression, long cycle lengths, and high volume-to-capacity ratios. Individual cycle failures are frequent occurrences.	There are typically long queues of vehicles waiting upstream of the intersection.	>55.0 and < 80.0	>35.0 and < 50.0
F	Forced Flow	Generally considered to be unacceptable to most drivers. Often occurs with over saturation. May also occur at high volume-to-capacity ratios. There are many individual cycle failures. Poor progression and long cycle lengths may also be major contributing factors.	Jammed conditions. Back- ups from other locations restrict or prevent movement. Volumes may vary widely, depending principally on the downstream back-up conditions.	> 80.0	> 50.0

References: 2010 Highway Capacity Manual

Peak hour traffic signal warrant analysis is completed for deficient intersections which are currently two-way or all-way stop controlled. The term "signal warrants" refers to the list of established criteria used by Caltrans and other public agencies to quantitatively justify or ascertain the need for installation of a traffic signal at an otherwise unsignalized intersection. This study has employed the signal warrant criteria presented in the latest edition of the Federal Highway Administration's (FHWA) Manual on Uniform Traffic Control Devices (MUTCD), as amended by the MUTCD 2014 California Supplement, for all study intersections. The signal warrant criteria are based upon several factors including volume of vehicular and pedestrian traffic, frequency of accidents, location of school areas etc. Both the FHWA's MUTCD and the MUTCD 2014 California Supplement indicate that the installation of a traffic signal should be considered if one or more of the signal warrants are met. The ultimate decision to signalize an intersection should be determined after careful analysis of all intersection and area characteristics. This traffic study specifically utilizes the Peak-Hour-Volume based Warrant 3 as one representative type of traffic signal warrant analysis. Warrant 3 criteria are essentially identical for both the FHWA's MUTCD and the MUTCD 2014 California Supplement. Since Warrant 3 provides specialized warrant criteria for intersections with rural characteristics (e.g. located in communities with populations of less than 10,000 persons or with adjacent major streets operating at above 40 mph), study intersections which use this specialized criteria are clearly identified. For the purposes of this study, the warrant criteria for intersections with urban characteristics are used and are included in the Appendix as necessary.

Table 4 presents the LOS thresholds for the automobile mode on urban street segments.

TABLE 4: AUTOMOBILE SEGMENT LOS CRITERIA

Travel Speed as a Percentage of Base Free	•	me-to-Capacity atio _a
Flow Speed (%)	≤1	≥1
>85	Α	F
>67-85	В	F
>50-67	С	F
>40-50	D	F
>30-40	E	F
≤30	F	F

a. Volume-to-Capacity Ratio of through movement at downstream boundary intersction

Table 5 presents the pedestrian LOS thresholds, and Table 6 presents the LOS thresholds for bicycles and transit.

TABLE 5: PEDESTRIAN LOS CRITERIA

	LOS by Average Pedestrian Space (ft²/p)					
Ped LOS Score	>60	>40-60	>24-40	>15-24	>8.0-15 ^a	<8.0 ^a
≤2.00	Α	В	С	D	E	F
>2.00-2.75	В	В	С	D	E	F
>2.75-3.5	С	С	С	D	E	F
>3.5-4.25	D	D	D	D	E	F
>4.25-5.00	Е	Е	Е	Е	E	F
>5.00	F	F	F	F	F	F

Notes: 1. Based on Highway Capacity Manual, Fifth Edition, Transportation Research Board, 2010.

TABLE 6: BICYCLE AND TRANSIT LOS CRITERIA

LOS Criteria for Bio	LOS Criteria for Bicycle and Transit Modes		
LOS	LOS SCORE		
Α	≤2.00		
В	>2.00-2.75		
С	>2.75-3.5		
D	>3.50-4.25		
E	>4.25-5.00		
F	>5.00		

Notes:

Table 7 presents the LOS thresholds for the basic freeway segments.

TABLE 7: BASIC FREEWAY SEGMENTS LOS CRITERIA

Segment Type			Density (pc/mi/ln)		
	A	В	C	D	E
Freeway	11	18	26	35	45
Merge	10	20	28	35	45
Diverge	10	20	28	35	45
Weave	10	20	28	35	45

Notes: 1. Based on Highway Capacity Manual, Fifth Edition, Transportation Research Board, 2010.

Technical Analysis Parameters

The following section details the multimodal technical analysis parameters. All parameters not listed should be assumed as default values or calculated based on parameters listed. The City maintains "Master" Synchro networks for the AM, Midday (MID), and PM peak hours which have be used to develop the project Synchro files for the AM and PM peak hour study periods. Modifications to reflect existing condition have been made as necessary and documented.

^{2.} All volume thresholds are approximate and assume ideal roadway characteristics. Actual thresholds for each LOS listed above may vary depending on a variety of factors including (but not limited to) roadway curvature and grade, intersection or interchange

^a In Cross-Flow situations, the LOS E/F threshold is 13 ft²/p

^{1.} Based on <u>Highway Capacity Manual, Fifth Edition</u>, Transportation Research Board, 2010.

^{2.} Also used for Ped and Bike LOS for intersections analysis

Vehicular Parameters

Synchro 9 (*Trafficware*) has been used to implement HCM 2010 analysis methodologies for vehicular delay at the study intersections. For signalized and all-way stop-controlled intersections, LOS determination is based on the calculated average delay for all approaches and movements. For a two-way stop-controlled intersection, an LOS determination is based upon the calculated average delay for all movements of the worst performing approach. SimTraffic software has been used to conduct the 95th percentile queuing analysis based on 5 runs of 60 simulation minutes with a seeding time of 15 minutes. The queues have been evaluated for impacts to turning pocket storage capacity and closely spaced intersections. Table 8 presents the main technical parameter assumptions that have been used in Synchro 9. Any parameters not included in Table 8 use the values included in the City's "Master" network or software default values.

TABLE 8: VEHICULAR TECHNICAL LOS PARAMETERS

Parameter	Assumption
1. Peak Hour Factor	Intersection Overall, based on existing counts
2. Heavy Vehicle Percentage	2%, except to/from US 101 Ramps will us 10%
3. Signal Timings	City 'Master' Synchro Networks (Unless otherwise noted)
4. Grades	2 percent of less for all intersections

The cycle lengths and signal timings for the study intersections were not changed from the City's "Master" Synchro networks except at the following intersections: Higuera Street/Prado Road and Higuera Street/Tank Farm Road. The Synchro "Master Network" for these intersections has signal phasing that does not reflect the existing conditions, based on aerial imagery. The intersection of Higuera Street/Prado Road is changed to have a 91-second cycle length with concurrent (dual-entry) phasing eastbound/westbound Prado Road with permitted left turns, instead of a split phase. The intersection of Higuera Street/Tank Farm Road is changed to have a 115-second cycle length with split phasing eastbound/westbound instead of a concurrent phase (permitted lefts) due to the dual left turns with shared through movement westbound, and a northbound right turn overlap phase is added.

In addition, the intersection of Higuera Street/Madonna Street phasing is changed to have the southbound right turn be protected plus an overlap phase, with no right turn allowed on red, and the eastbound right turn is permitted plus overlap phasing. At the intersection of Higuera Street/South Street, PM peak hour phasing is changed to be consistent with the AM peak hour with the addition of a northbound right turn overlap phase. At the intersections of Los Osos Valley Road and Calle Joaquin, and at both US 101 Northbound and Southbound Ramp terminals, the main street northbound/southbound left turn phasing is changed to be permitted plus protected, and at Los Osos Valley Road/US 101 Northbound Ramps the southbound right turn is changed to include an overlap phase. These changes were made to the City's Synchro networks in order to reflect existing conditions.

Vehicular segment analysis has been conducted manually using the equations within HCM 2010. Omni-Means has created a spreadsheet which formulates the HCM 2010 equations within Chapter 17: *Urban Street Segments*. Table 9 presents critical technical parameters required and our assumptions. Any parameters not included in Table 9 use default values.

TABLE 9: VEHICULAR TECHNICAL LOS PARAMETERS FOR SEGMENT ANALYSIS

Parameter	Assumption
1. Lane Width	Measured from Aerials
2. Median Type	Identified from Aerials
3. Total Access Points	Identified from Aerials
4. Number of Travel Lanes	Identified from Aerials

Pedestrian Parameters

Intersection pedestrian LOS has also been determined using Synchro 9. Synchro 9 uses HCM 2010 methodologies for determining pedestrian LOS, and requires technical inputs beyond those included for vehicular LOS. Table 10 presents critical technical parameters required and our assumptions. Any parameters not included in Table 10 use software default values.

TABLE 10:
PEDESTRIAN TECHNICAL LEVEL OF SERVICE PARAMETERS FOR INTERSECTION ANALYSIS

Parameter	Assumption
1. Right Corner Size A, Size B, and Curb Radius	Measured from Aerials
2. Number of Right-Turn Islands	Identified from Aerials
3. Crosswalk Widths	Default Value of 10 feet
4. Ped Left-Right Flow Rate	Half of Two-Way Flow Rate
5. Ped Right-Left Flow Rate	Half of Two-Way Flow Rate
6. Ped R Sidewalk Flow Rate	Same as Crossing Volume
7. Vehicle Perm Left Flow in Ped Phase	Based on SimTraffic Observation
8. Vehicle Perm Right Flow in Ped Phase	Based on SimTraffic Observation
9. Vehicle Right Turn on Red	Based on SimTraffic Observation

For segment analysis, pedestrian LOS has been determined using HCM 2010. Table 11 presents critical technical parameters required and our assumptions. Any parameters not included in Table 11 use software default values.

TABLE 11: PEDESTRIAN TECHNICAL LEVEL OF SERVICE PARAMETERS FOR SEGMENT ANALYSIS

Parameter	Assumption
1. Two-Way Ped Volume	From Counts
2. Ped Waiting Delay	Default
3. Pedestrian Free-Flow Speed	3.5 ft/sec
4. Width and Number of Vehicle and Bike Lanes	Measured from Aerials
5. Sidewalk Presence	Identified from Aerials
6. Inside Fixed-Object Effective Width	Measured in Field
7. Outside Fixed-Object Effective Width	Measured in Field
8. Buffer Width	Measured in Field
9. Nearest Signal Distance	Measured from Aerials
10. Sidewalk Length Adjacent to Window,	
Building, Fence	Measured from Aerials
11. Propotion of Parking Occupied	Measured from Aerials
12. Length of Segment	Measured from Aerials
13. Intersection Width	Measured from Aerials

Bicycle Parameters

Intersection bicycle LOS has also been determined using Synchro 9. Synchro 9 uses HCM 2010 methodologies for determining bicycle LOS, and requires technical inputs beyond those included for vehicular and pedestrian LOS. Table 12 presents critical technical parameters required and our assumptions. Any parameters not included in Table 12 use software default values.

TABLE 12: BICYCLE TECHNICAL LEVEL OF SERVICE PARAMETERS FOR INTERSECTION ANALYSIS

Parameter	Assumption	
1. Bike Flow Rate	From Counts	
2. Bike Lane Width	Measured from Aerials	
3. Paved Shoulder Width	Measured from Aerials	
4. Curb is Present	Identified from Aerials	
5. On Street Parking	Identified from Aerials	

For segment analysis, bicycle LOS has been determined using HCM 2010. Table 13 presents critical technical parameters required and our assumptions. Any parameters not included in Table 13 use software default values.

TABLE 13: BICYCLE TECHNICAL LEVEL OF SERVICE PARAMETERS FOR SEGMENT ANALYSIS

Parameter	Assumption		
1. Bicycle Running Speed	Default, 15 mph		
2. Percent Heavy Vehicle	2%		
3. Pavement Condition Rating	3.0		

Transit Parameters

Transit LOS is determined for segments using the equations within the Urban Street Segments chapter of HCM 2010; Synchro software does not analyze transit routes or stops. The transit LOS is determined for the stop for Route 2 at the intersection of Tank Farm Road and S. Higuera Street. Route 2 stops on the far side of the intersection, headed eastbound on Tank Farm Road, and a bus turnout is present (off-line stop). Table 14 presents critical technical parameters required and our assumptions. Any parameters not included in Table 14 use default values.

TABLE 14:
TRANSIT TECHNICAL LEVEL OF SERVICE PARAMETERS FOR SEGMENT ANALYSIS

Parameter	Assumption	
Average Passenger Load Factor	Identified from Short Range Transit Plan Weekday Riderships	
2. Average Dwell Time	Default, 20 sec	
3. Reentry Delay	Default, 5 sec	
4. Bus Frequency/Headways	Identified from Bus Schedules	
5. Acceleration/Deceleration Rate	Default, 4.0 ft/s ²	
6. Transit Performance	Identified from Short Range Transit Plan	
7. Stops with shelters or benches	Identified from Aerials	
8. Average Passenger Trip Length	3.7 mi	

Highway Segments Parameters

Vehicular LOS along the freeway segments (US 101) is determined using the Highway Capacity Software (HCS 2010, version 6.7), which uses HCM 2010 methodologies. Table 15 presents critical technical parameters required and our assumptions. Any parameters not included in Table 15 use default values.

TABLE 15: HCS PARAMETERS FOR HIGHWAY SEGMENT ANALYSIS

Parameter	Assumption	
Volume	Caltrans 2014 Data	
Heavy Vehichle Percentage	From Caltrans Data, 10%	
Free-Flow Speed	Speed Limit identified from Aerials	
Terrain	Level	
Peak Hour Factor	0.92	

Data Collection

Twenty (20) intersections have been identified by the City of San Luis Obispo for analysis. City-provided counts were conducted in February and March of 2014.

1. N	Madonna Road and Los Osos Valley Road		City-provided traffic counts
2. N	Madonna Road and Oceanaire Drive	City-provided &	Omni-Means traffic counts
3. N	Madonna Road and Dalidio Drive		City-provided traffic counts
4. N	Madonna Road and El Mercado		City-provided traffic counts
5. N	Madonna Road and US 101 Southbound Ram	nps	City-provided traffic counts
	Madonna Road and US 101 Northbound Ram	•	City-provided traffic counts
	Madonna Road and Higuera Street	•	City-provided traffic counts
	Higuera Street and South Street		City-provided traffic counts
	∟os Osos Valley Road and Froom Ranch Way	1	City-provided traffic counts
	os Osos Valley Road and Auto Park Way		City-provided traffic counts
	os Osos Valley Road and Calle Joaquin		City-provided traffic counts
	os Osos Valley Road and US 101 Southbour	nd Ramps	City-provided traffic counts
13. L	∟os Osos Valley Road and US 101 Northboun	d Ramps	City-provided traffic counts
	Los Osos Valley Road and S. Higuera Street	·	City-provided traffic counts
	S. Higuera Street and Suburban Drive		City-provided traffic counts
	S. Higuera Street and Tank Farm Road		City-provided traffic counts
	S. Higuera Street and Granada Drive		City-provided traffic counts
18. 5	S. Higuera Street and Prado Road		City-provided traffic counts
	S. Higuera Street and Margarita Avenue		City-provided traffic counts
	Prado Road and US 101 Northbound Ramps/l	Elks Lane	Omni-Means traffic counts

Omni-Means has also collected counts for the entrance driveway for the US Post Office driveway/Promenade on Dalidio Drive, which is approximately 270 feet south of the intersection of Madonna Road and Dalidio Drive. This intersection has been analyzed as part of the access management for the proposed project.

Ten (10) arterial roadway segments have been identified and pre-selected by the City of San Luis Obispo for analysis. However, some of these segments are required to be broken up into separate segments in between traffic signals for analysis. Average Daily Traffic (ADT) counts were provided by the City for the segments.

Madonna Road:

- 19. Los Osos Valley Road to Oceanaire Drive
- 20. Oceanaire Drive to Dalidio Drive
- 21. Dalidio Drive to El Mercado
- 22. El Mercado to US 101 Southbound Ramps
- 23. US 101 Southbound Ramps to US 101 Northbound Ramps
- 24. US 101 Northbound Ramps to Higuera Street

Higuera Street:

- 25. Madonna Road to Margarita Avenue
- 26. Margarita Avenue to Prado Road
- 27. Prado Road to Granada Drive
- 28. Granada Drive to Tank Farm Road
- 29. Tank Farm Road to Suburban Road
- 30. Suburban Road to Los Osos Valley Road

Los Osos Valley Road:

- 31. Madonna Road to Froom Ranch Way
- 32. Froom Ranch Way to Calle Joaquin
- 33. Calle Joaquin to US 101 Southbound Ramps
- 34. US 101 Southbound Ramps to US 101 Northbound Ramps
- 35. US 101 Northbound Ramps to S. Higuera Street

Prado Road

1. US 101 Northbound Ramps to Higuera Street

Dalidio Drive

2. Madonna Road to Froom Ranch Way (Future)

Froom Ranch Way

3. Los Osos Valley Road to Dalidio Drive (Future)

Two highway segments have been identified and pre-selected by the City of San Luis Obispo and Caltrans for analysis. Average Daily Traffic (ADT) counts were provided by Caltrans for the segments.

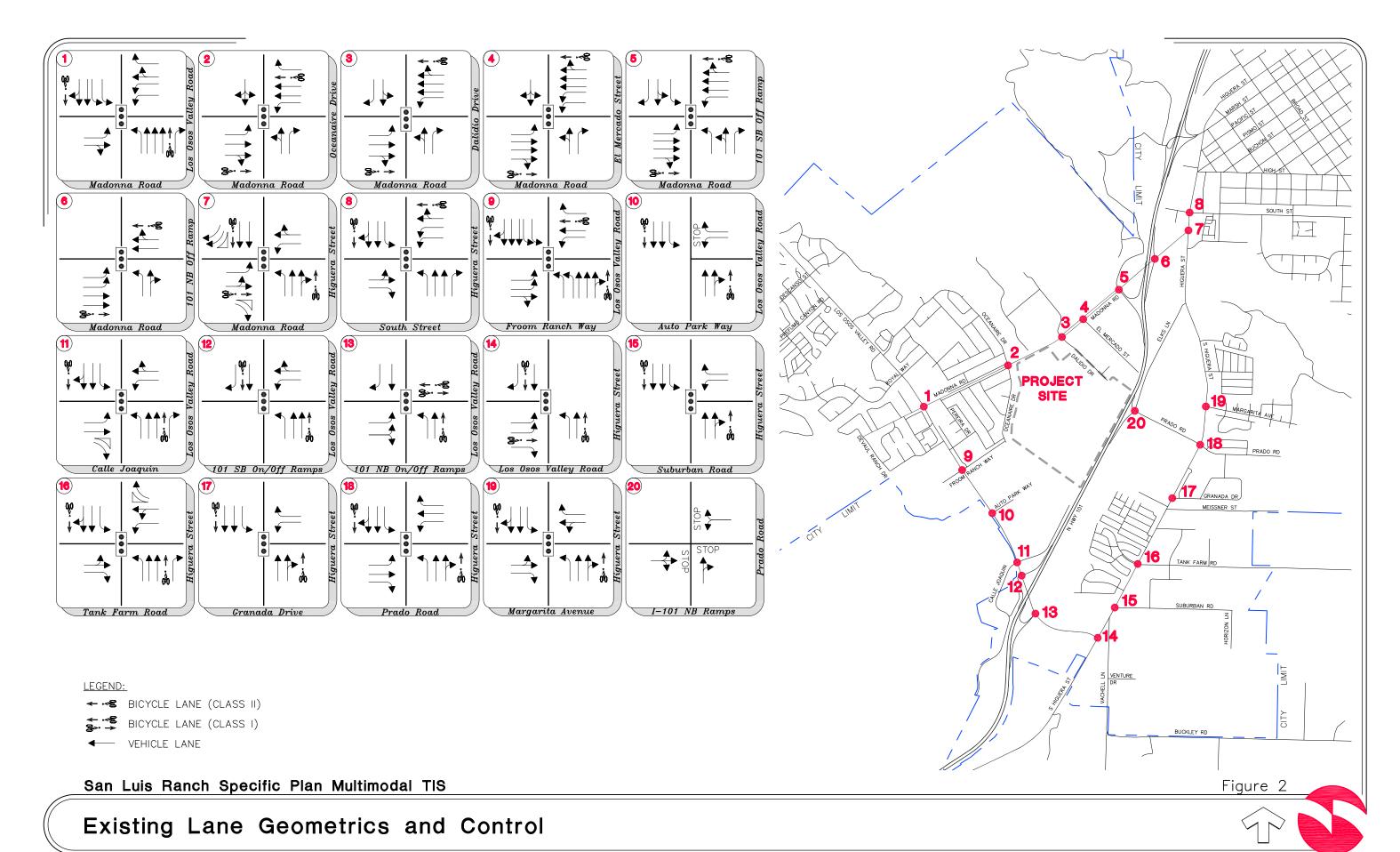
The two segments are as follows:

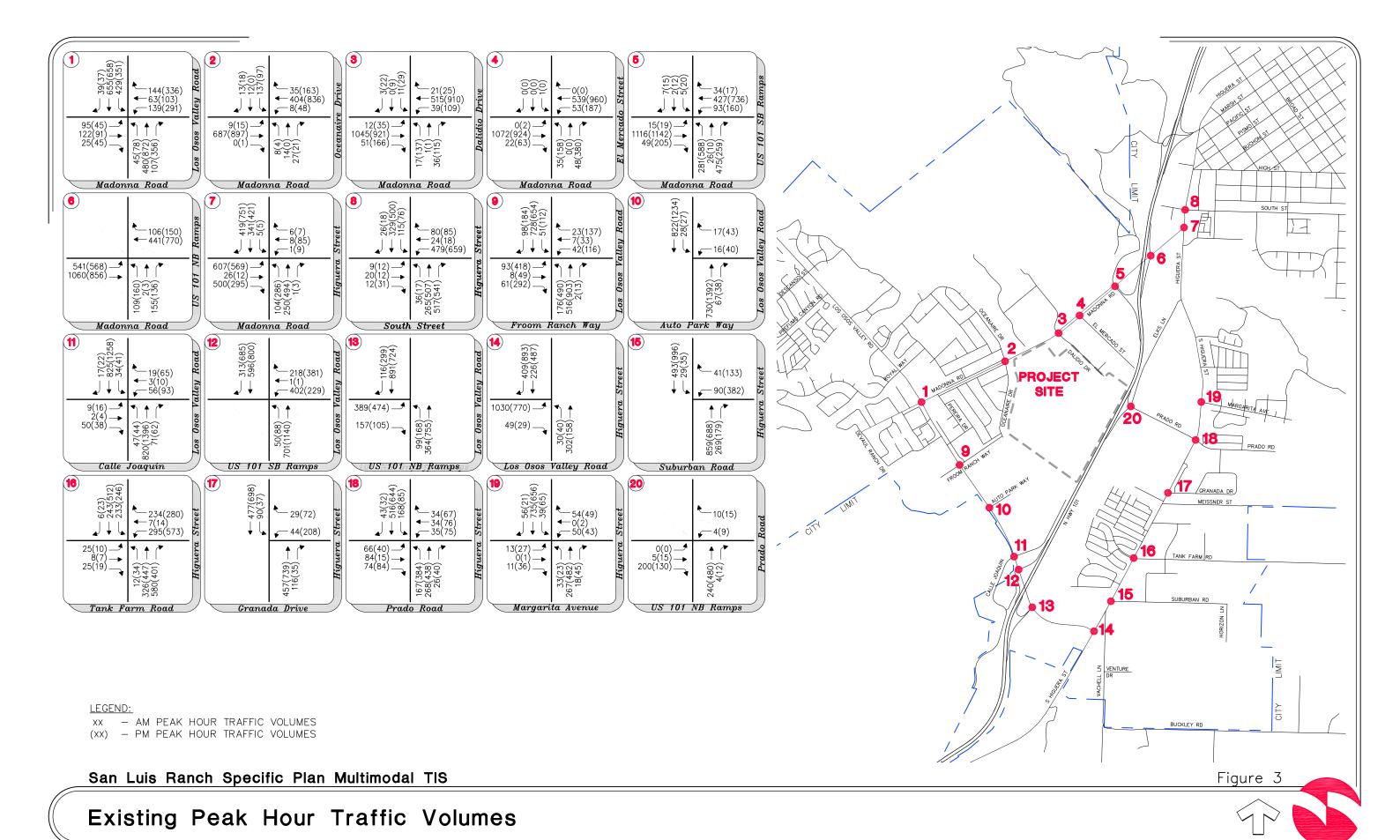
- 3. US 101 between Los Osos Valley Road and Prado Road
- 4. US 101 between Prado Road and Madonna Road

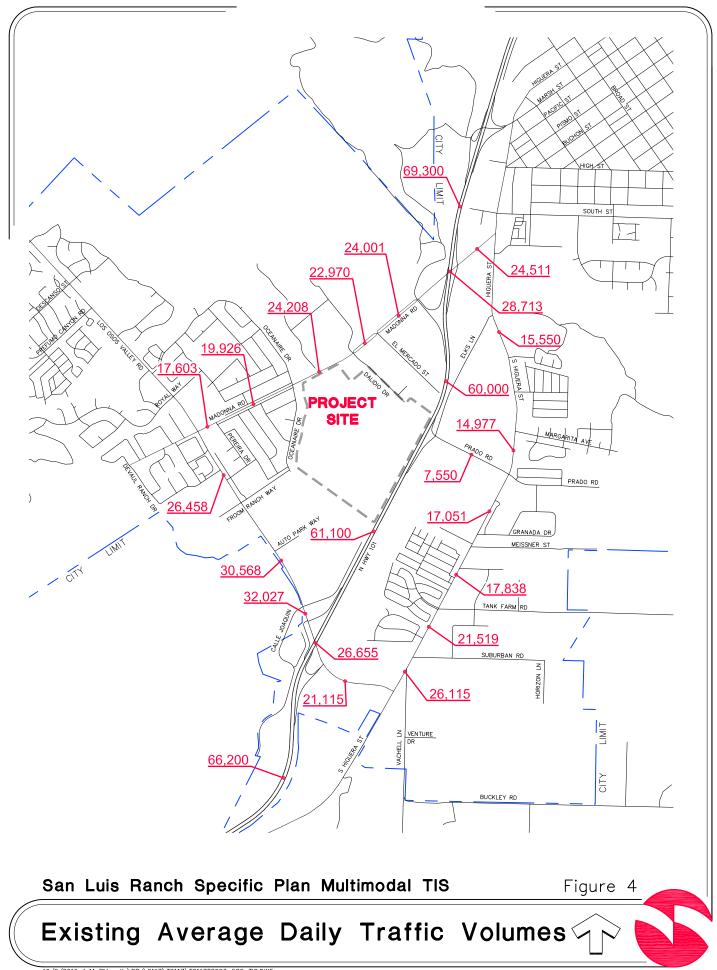
For the above locations, available AM and PM peak hour vehicular, bicycle, and pedestrian counts were obtained from either the City or collected by Omni-Means.

Existing Conditions

Existing conditions establish baseline traffic conditions that currently exist in the study area. Currently, the study area is located along the S. Higuera Street corridor between Prado Road and Los Osos Valley Road, which is developed with housing in the southwest area, commercial and offices fronting Prado Road and S. Higuera Street on the east, and service and manufacturing land use along Tank Farm Road and south of Suburban Road. Existing turning movement count data was provided by the City of San Luis Obispo, or collected by Omni-Means, for weekday AM and PM peak hours, along with the City-maintained "Master" Synchro networks which are used to develop the project Synchro files. The counts were taken from the City's Traffic Counts and Speed Surveys database (online). Figure 2 presents the study locations and intersection lane geometries. Figure 3 presents the existing peak hour volumes at the study intersections. Figure 4 presents the Average Daily Traffic (ADT) along the street segments.







Roadway Network

The study roadways for this project are described below. Due the varying nature of the City street network the directionality defined below may or may not be consistent with how directionality has been presented in other traffic studies or documentation, this however does not affect the results of the analysis and findings.

<u>US Highway 101 (US 101)</u> is functionally classified as Urban Principal Arterial and is part of the National Truck Network. Land uses along US 101 are comprised of urban commercial and residential, open space, national forest and grazing land. US 101 is a north-south, four lane mixed flow freeway through the City of San Luis Obispo. Between several closely spaced interchanges north of Los Osos Valley Road, auxiliary lanes are provided to facilitate the flow of entering and exiting traffic. Outside the City, US 101 provides access to Paso Robles to the north and the Five Cities area to the south. It is a primary route for all truck traffic originated from/destined to the City of San Luis Obispo (SLO LUCE, 2014).

Froom Ranch Way is a east-west roadway that connects Los Osos Valley Road to the Perfumo Creek Shopping Center on the east side and the Irish Hills Plaza on the west side. These large shopping centers contain stores such as Costco Wholesale, Home Depot, Whole Foods, BevMo!, Target, etc. that attract a large number of trips and is located in between the US 101 interchange and large residential neighborhoods. Froom Ranch Way is four lanes west of Los Osos Valley Road and two lanes east of Los Osos Valley Road with sidewalks on both sides and a Class II bike lane on the east side. The speed limit is 25 mph. Planned changes to Froom Ranch Way include extension as a two-lane collector east to Dalidio Drive

<u>Higuera Street</u> (S. Higuera Street) is a north-south arterial within the study area. Higuera Street connects to downtown San Luis Obispo to the north and terminates to the south at its interchange with US 101. South of Marsh Street, it provides a four-lane roadway with Class II bike lanes and continuous sidewalks. The posted speed limit within the study area is 45 mph.

<u>Los Osos Valley Road</u> is a north-south arterial that extends from Los Osos to Higuera Street. It connects with US 101 ramp termini approximately 1/2 mi west of S. Higuera Street and is functionally classified in the City as an Arterial or a Parkway Arterial. It provides four lanes with Class II bike lanes and sidewalks on both sides for the majority of the study area. The posted speed limit within the study area is 35 - 45 mph.

Since data was collected in 2014, improvements due to the Los Osos Valley Road (LOVR)/US 101 Interchange Traffic Relief Project have been constructed. Improvements included widening Los Osos Valley Road to four lanes from the existing four-lane section west of Calle Joaquin Road to approximately 500 feet west of South Higuera Street; and constructing sidewalks and Class II bicycle lanes along both sides of Los Osos Valley Road.

<u>Madonna Road</u> is an east-west roadway that extends from Devaul Ranch Road just west of Los Osos Valley Road and ends at Higuera Street to the east. It is functionally classified as a local street west of Los Osos Valley Road and an arterial east of Los Osos Valley Road. Madonna Road provides a total of two, four or six travel lanes, and Class II bike lanes with sidewalks on one or both sides. It also connects with US 101 ramp termini approximately 1/2 mile west of Higuera Street. The posted speed limit within the study area is 40 mph.

<u>Prado Road</u> is an east-west two-lane roadway that extends eastward from the US 101 Northbound Ramps and ends east of Higuera Street. It is functionally classified as a regional route/highway in the City's General Plan. Planned changes to Prado Road include extension west from US 101 to Madonna Road and east to Broad Street. Prado Road is a two-lane roadway with sidewalks on both sides and on-street parking at various locations. The posted speed limit within the study area is 35 mph.

<u>Tank Farm Road</u> is an east-west parkway arterial which connects S. Higuera Street to Broad Street to the east, and continues as Orcutt Road east of the Orcutt Area. In the study area, it is a four-lane roadway with Class II bike lanes and sidewalks on both sides. The posted speed limit within the study area is 45 mph.

Existing Conditions Analysis

The Existing conditions multimodal analysis for the study intersections and segments are presented below.

Existing Conditions Intersection Analysis

Table 16 provides a summary of the Existing conditions vehicular AM and PM peak hour intersection delay and LOS. Table 17 provides a summary of the Existing pedestrian AM and PM peak hour conditions at the study intersections. Table 18 provides a summary of the Existing bicycle AM and PM peak hour conditions at the study intersections. Table 19 provides a summary of the Existing conditions queuing analysis.

TABLE 16: EXISTING CONDITIONS INTERSECTION LEVEL OF SERVICE: AUTOMOBILE ANALYSIS

				Α	M Peak F	lour	PN	/ Peak Ho	our
#	Intersection	Control Type ^{1,2}	Target LOS	v/c³	Delay	LOS	v/c³	Delay	LOS
1	Madonna Road/Los Osos Valley Road	Signal	D		26.1	С		45.0	D
2	Madonna Road/Oceanaire Drive	Signal	D		19.6	В		14.2	В
3	Madonna Road/Dalidio Drive	Signal	D		9.7	Α	2.11	56.2	Е
4	Madonna Road/El Mercado	Signal	D		7.3	Α		19.6	В
5	Madonna Road/US 101 SB Ramps/Madonna	Signal	С	1.31	48.7	D		23.4	С
6	Madonna Road/US 101 NB Ramps	Signal	С		17.4	В		21.1	С
7	Madonna Road/Higuera Street	Signal	D		18.6	В		21.7	С
8	Higuera Street/South Street	Signal	D		21.4	С	1.31	63.1	E
9	Los Osos Valley Road/Froom Ranch Way	Signal	D		19.4	В		34.6	С
10	Los Osos Valley Road/Auto Park Way	TWSC	D		15.0	В		34.1	D
11	Los Osos Valley Road/Calle Joaquin	Signal	D		4.6	Α		5.6	Α
12	Los Osos Valley Road/US 101 SB Ramps	Signal	С		13.0	В		19.0	В
13	Los Osos Valley Road/US 101 NB Ramps	Signal	С		27.6	С		21.8	С
14	S. Higuera Street/Los Osos Valley Road	Signal	D		16.0	В		19.1	В
15	S. Higuera Street/Suburban Drive	Signal	D		6.3	Α		11.1	В
16	S. Higuera Street/Tank Farm Road	Signal	D		36.2	D		21.0	С
17	S. Higuera Street/Granada Drive	Signal	D		8.4	Α		10.5	В
18	S. Higuera Street/Prado Road	Signal	D		16.7	В		20.9	С
19	S. Higuera Street/Margarita Avenue	Signal	D		7.4	Α		10.8	В
20	Prado Road/US 101 NB Ramps	AWSC	С		9.0	Α		13.5	В

Notes:

^{1.} AWSC = All Way Stop Control; TWSC = Two Way Stop Control; RNDBT = Roundabout

^{2.} LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC, Signal, RNDBT

^{3.} Volume to Capacity Ratio (v/c) is for worst movement delay, for unacceptable LOS only

TABLE 17: EXISTING CONDITIONS INTERSECTION LEVEL OF SERVICE: PEDESTRIAN ANALYSIS

Į				AM Peak Ped.	nour	PM Peak Ped.	Hour
ı	Intersection	Approach	Target LOS	Crosswalk Score	LOS	Crosswalk Score	LO
1		EB	C	2.10	В	2.11	В
ı	Madonna Road/Los Osos Valley Road	WB	С	2.90	С	3.16	С
ı	iviadoriria Road/Los Osos Valley Road	NB	С	2.94	С	3.41	С
		SB	С	3.28	С	3.18	С
ı		EB	C	2.69	В	2.87	C
ı	Madonna Road/Oceanaire Drive	WB	С	3.05	C	3.28	С
ı	Maderina Medal e e e aname E me	NB	С	1.99	Α	2.07	В
		SB	С	1.86	A	1.85	A
ı		EB	С	2.96	C	3.24	C
ı	Madonna Road/Dalidio Drive	WB NB	C	2.99 2.06	C B	3.07 2.25	C B
ı			C				
4		SB		1.98	A	2.03	В
ı		EB WB	C	n/a 3.07	- C	n/a 3.16	- C
ı	Madonna Road/El Mercado	NB	C	2.26	В	2.75	В
ı		SB	C	1.74	A	1.74	A
1		EB	C	3.00	C	3.16	C
ı	Madonna Road/US 101 SB Ramps/Madonna	WB	C	n/a	-	n/a	-
ı	Inn	NB	Č	2.75	В	2.67	В
ı	""1	SB	C	2.17	В	2.18	В
١		EB	C	n/a		2.10 n/a	
		WB	C	2.84	Ċ	2.80	c
ı	Madonna Road/US 101 NB Ramps	NB	C	1.99	A	2.04	В
		SB	C	n/a	- ' '	n/a	-
١		EB	C	3.01		2.91	
		WB	C	1.98	A	2.91	A
	Madonna Road/Higuera Street	NB	C	2.70	В	2.78	Ĉ
		SB	C	n/a	-	n/a	-
1		EB	C	2.01	В	2.01	В
ı		WB	č	2.73	В	2.77	Č
ı	Higuera Street/South Street	NB	Č	n/a	-	n/a	-
ı		SB	С	2.48	В	2.54	В
1		EB	C	2.49	В	2.84	C
ı		WB	Č	2.38	В	2.59	В
ı	Los Osos Valley Road/Froom Ranch Way	NB	C	n/a	-	n/a	-
ı		SB	С	3.06	С	3.26	С
1		EB	С		<u> </u>	0.20	
ı		WB	č		-		-
ı	Los Osos Valley Road/Auto Park Way	NB	С	n/a	-	n/a	-
ı		SB	С		-		-
1		EB	С	2.48	В	2.27	В
ı		WB	C	2.08	В	2.16	В
ı	Los Osos Valley Road/Calle Joaquin	NB	С	2.98	С	3.19	С
ı		SB	С	2.88	С	3.17	С
1		EB	С	1.90	Α	2.32	В
ı		WB	С	2.23	В	2.12	В
ı	Los Osos Valley Road/US 101 SB Ramps	NB	С	n/a	-	n/a	-
ı		SB	С	n/a	-	n/a	-
1		EB	С	2.44	В	2.58	В
ı	Los Osos Valley Road/US 101 NB Ramps	NB	С	n/a	-	n/a	-
	,,	SB	С	n/a	-	n/a	-
١		EB	C	2.65	В	2.73	В
ı	S. Higuera Street/Los Osos Valley Road	NB	Č	2.25	В	2.30	В
		SB	С	n/a	-	n/a	-
١		WB	C	2.15	В	2.29	В
	S. Higuera Street/Suburban Drive	NB	č	2.98	Č	3.60	Ď
J		SB	Č	2.73	В	2.85	С
1		EB	С	2.01	В	2.01	В
J	S. Higuera Street/Tank Farm Road	WB	С	2.91	С	3.04	С
		NB	С	3.33	С	3.27	C
J		SB	C	2.65	B	2.78	C
	C 15 Ct+/C	WB	С	2.05	В	2.13	В
	S. Higuera Street/Granada Drive	NB	С	n/a	-	n/a	-
ł		SB	C	2.60	В	2.76	C
		EB	С	2.35	В	2.41	В
Į	S. Higuera Street/Prado Road	WB	С	2.28	В	2.31	В
Į	*	NB	С	2.71	В	2.95	С
J		SB	С	2.76	С	2.78	С
ı		EB	O	2.22	В	2.06	В
	S. Higuera Street/Margarita Avenue	WB	С	2.12	В	2.17	В
ļ		NB	С	2.74	В	2.79	С
		SB	С	2.70	В	2.77	С
		EB	С		-		-
	Prado Road/US 101 NB Ramps	EB WB NB	CCC	n/a	-	n/a	-

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective.

HCM 2010 Methodologies for the pedestrian mode at two-way stop-controlled intersections is limited to the uncontrolled crossing. No methodology exists for evaluating pedestrian performance for the stop controlled approach (cross-street). However, it is reasoned that this type of control has negligible influence on pedestrian service along the segment.

TABLE 18: EXISTING CONDITIONS INTERSECTION LEVEL OF SERVICE: BICYCLE ANALYSIS

			Target	AM Pea	k Hour	PM Pea Bicycle LOS	k Hour
#	Intersection	Approach	LOS	Score	LOS	Score	LOS
		EB	D	3.26	С	3.14	С
		WB	D	3.37	С	3.99	D
	Madonna Road/Los Osos Valley Road	NB	D	1.64	Α	2.01	В
		SB	D	2.60	В	2.49	В
		EB	D	2.72	В	2.91	C
		WB	D	1.05	A	1.59	A
	Madonna Road/Oceanaire Drive		D				
		NB		2.74	В	2.70	В
		SB	D	2.22	B	2.13	<u>B</u>
		EB	D	2.17	В	2.08	В
3	Madonna Road/Dalidio Drive	WB	D	1.47	Α	1.71	Α
	madernia redau Banale Brive	NB	D	2.99	С	3.32	С
		SB	D	2.84	С	2.92	С
		EB	D	1.80	Α	1.67	Α
		WB	D	1.67	Α	1.94	Α
	Madonna Road/El Mercado	NB	D	3.19	С	3.93	D
		SB	D	3.03	C	3.03	С
		EB	D	2.00	A	2.05	В
	Madonna Road/US 101 SB Ramps/Madonna	WB	D	1.61	A	1.79	A
	Inn	NB	D		-		-
				n/a		n/a	
_		SB	D	2.90	C	2.96	C
		EB	D	2.69	В	2.33	В
	Madonna Road/US 101 NB Ramps	WB	D	1.58	Α	1.82	Α
		NB	D	n/a	-	n/a	-
		EB	D	3.27	С	2.75	В
	Madonna Road/Higuero Stroot	WB	D	2.43	В	2.58	В
	Madonna Road/Higuera Street	NB	D	1.69	Α	2.05	В
		SB	D	2.14	В	2.48	В
		EB	D	2.70	В	2.73	В
		WB	D	2.59	В	2.94	C
	Higuera Street/South Street	NB	D	2.94	C	3.18	c
		SB	D	1.53	A	1.66	A
		EB	D	3.38	C	4.39	E
,	Los Osos Valley Road/Froom Ranch Way	WB	D	1.81	Α	2.49	В
	· · · · · · · · · · · · · · · · · · ·	NB	D	1.73	Α	2.12	В
		SB	D	1.72	Α	1.74	A
		EB	D		-		-
	Las Casa Vallari Basal/Arita Baski Mari	WB	D	/	-	/	-
J	Los Osos Valley Road/Auto Park Way	NB	D	n/a	-	n/a	-
		SB	D		-		-
		EB	D	2.99	С	2.97	С
		WB	D	3.08	С	3.22	С
1	Los Osos Valley Road/Calle Joaquin	NB	D	1.45	Α	1.87	A
		SB	D	0.37	A	0.69	A
		WB	D	n/a	-	n/a	
	L O V-II B I/I IO 404 OB B					1	
_	Los Osos Valley Road/US 101 SB Ramps	NB	D	2.69	В	3.04	С
		SB	D	2.33	В	3.18	С
		EB	D	n/a	-	n/a	-
3	Los Osos Valley Road/US 101 NB Ramps	NB	D	1.82	Α	2.58	В
		SB	D	3.45	С	3.43	С
		EB	D	1.99	Α	1.73	Α
4	S. Higuera Street/Los Osos Valley Road	NB	D	1.87	Α	1.63	Α
		SB	D	2.38	В	3.67	D
		WB	D	0.89	A	1.55	A
5	S. Higuera Street/Suburban Drive	NB	D	2.20	В	1.94	Α
	5	SB	D	1.71	A	2.13	В
		EB	D	2.70	B	2.13	В
3	S. Higuera Street/Tank Farm Road	WB	D	2.45	В	2.99	С
		NB	D	2.07	В	2.01	В
		SB	D	1.66	A	1.91	A
		WB	D	2.63	В	2.99	С
7	S. Higuera Street/Granada Drive	NB	D	1.70	Α	1.88	Α
		SB	D	1.83	Α	1.98	Α
		EB	D	2.37	В	2.22	В
	C. Historia Street/Dred - D	WB	D	2.69	В	2.90	С
3	S. Higuera Street/Prado Road	NB	D	1.64	Α	1.99	Α
		SB	D	1.87	Α	1.90	Α
		EB	D	2.46	В	2.53	В
		WB	D	2.69	В	2.68	В
9	S. Higuera Street/Margarita Avenue		D				
		NB ep		1.57	A	1.78	A
	<u> </u>	SB	D	2.11	В	2.04	В
		EB	D		-	1	-
S	Prado Road/US 101 NB Ramps	WB	D	n/a	-	n/a	-
		NB	D		-	1	-
	l .						

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective.

No methodology exists for evaluating bicycle performance at two-way stop-controlled intersections. However, it is reasoned that this type of control has negligible influence on bicycle service along the segment for stop controll on the cross-street.

TABLE 19: EXISTING CONDITIONS 95TH PERCENTILE QUEUING ANALYSIS

	Intersection			Total	Queue/l	rcentile Lane (ft)
ID	Location	Movement	No. Lanes	Storage (ft) ¹	AM Peak Hour	PM Peak Hour
1	Madonna Road/Los	Northbound Right	1	175	96	240
7	Madonna	Eastbound Right	1	150	232	146
,	Road/Higuera Street	Northbound Left	1	160	107	251
		Westbound Left	2	240	219	310
8	Higuera Street/South	Northbound Left	1	60	87	74
0	Street	Northbound Right	1	60	142	136
		Southbound Left	1	70	109	97
9	Los Osos Valley Road/Froom Ranch	Westbound Right	1	50	41	82
11	Los Osos Valley Road/Calle Joaquin	Southbound Left	1	180	108	170
	Los Osos Valley	Westbound Left	1	150	241	224
12	Road/US 101 SB	Northbound Left	1	80	114	122
	Ramps	Southbound Through	1	240	297	289
	Los Osos Valley	Eastbound Left/Right	1	200	221	177
13	Road/US 101 NB	Northbound Left	1	140	117	181
13	Ramps	Southbound Through	1	865	1042	822
	Ramps	Southbound Right	1	60	186	219
14	S. Higuera Street/Los Osos Valley Road	Eastbound Right	1	90	164	122
16	S. Higuera	Northbound Right	1	100	137	134
16	Street/Tank Farm	Southbound Left	1	165	187	218
18	S. Higuera	Northbound Left	1	100	131	176
10	Street/Prado Road	Southbound Left	1	60	116	109
19	S. Higuera	Southbound Left	1	60	58	67

Notes: 1. Bolded entries indicate queues exceed available storage

- Storage Length of " " represents a lane which exceeds 1,000 feet, usually a through lane.
 For Movements with more than one lane, the maximum of the 95th percentile queue is reported.
- 4. * Represents storage lengths for one lane; second lane is a left or right trap lane.

As shown in Table 16 above, the intersection of Madonna Road/Dalidio Drive, Madonna Road/US 101 SB Ramps/Madonna Inn, Higuera Street/South Street, and Los Osos Valley Road/Auto Park Way are currently operating at unacceptable conditions during AM and PM peak hours. The minor street northbound and southbound approaches experience long delays. Pedestrian and bicycle analysis shows acceptable conditions at study intersections, except for the intersections of Los Osos Valley Road/US 101 SB Ramps and Los Osos Valley Road/Froom Ranch Way.

Existing Conditions Segment Analysis

Table 20 provides a summary of the Existing vehicular AM and PM peak hour conditions for the study segments. Table 21 provides a summary of the Existing pedestrian AM and PM peak hour conditions for the study segments. Table 22 provides a summary of the Existing bicycle AM and PM peak hour conditions for the study segments. Table 23 provides a summary of the Existing transit AM and PM peak hour conditions for the study segments. Table 24 provides a summary of the Existing freeway segments analysis for AM and PM peak hour conditions for the study segments along US 101.

TABLE 20: EXISTING CONDITIONS SEGMENT LEVEL OF SERVICE: AUTOMOBILE ANALYSIS

AUTO	SEGMENT LOS							PEAK			PM P		
					LOS		Base Free-			Travel	Base Free-	Travel	
					Threshold	Travel Speed	Flow Speed	Travel Speed/		Speed	Flow Speed	Speed/	
ID	Roadway	From	То	Direction		(mph)	BFFS (mph)	BFFS (%)	LOS	(mph)	BFFS (mph)	BFFS (%)	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	D	21.4	42.1	51%	С	12.9	42.1	31%	E
	Madonna Rd	LOVR	Oceanaire Dr	EB	D	29.3	42.1	70%	В	25.7	42.1	61%	С
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	D	22.7	40.8	56%	С	19.2	40.7	47%	D
	Madonna Rd	Oceanaire Dr	Dalidio	EB	D	27.1	40.7	66%	С	18.9	40.8	46%	D
3	Madonna Rd	El Mercado	Dalidio Dr	WB	D	21.2	34.8	61%	С	14.1	34.8	41%	D
	Madonna Rd	Dalidio Dr	El Mercado	EB	D	21.2	34.7	61%	С	13.3	34.6	39%	E
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	32.2	37.9	85%	Α	21.2	37.3	57%	С
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	22.5	37.8	59%	С	18.6	37.7	49%	D
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	34.5	37.8	91%	Α	34.0	37.8	90%	Α
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	32.7	37.8	86%	Α	33.5	37.8	88%	Α
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	9.2	37.2	25%	F	10.6	37.2	29%	F
	Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	17.4	37.2	47%	D	13.3	37.2	36%	E
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	D	37.5	44.5	84%	В	36.0	44.5	81%	В
	S. Higuera St	Margarita Ave	Madonna Rd	NB	D	35.7	44.8	80%	В	36.8	44.8	82%	В
8	S. Higuera St	Margarita Ave	Prado Rd	SB	D	18.6	38.9	48%	D	16.5	38.9	42%	D
	S. Higuera St	Prado Rd	Margarita Ave	NB	D	26.3	38.9	68%	В	22.0	38.9	57%	С
9	S. Higuera St	Prado Rd	Granada Dr	SB	D	33.8	41.8	81%	В	30.6	41.8	73%	В
	S. Higuera St	Granada Dr	Prado Rd	NB	D	25.6	41.9	61%	С	28.1	41.9	67%	В
10	S. Higuera St	Granada Dr	Tank Farm Road	SB	D	42.6	41.6	102%	A	27.4	42.6	64%	C
	S. Higuera St	Tank Farm Road	Granada Dr	NB	D	30.5	41.6	73%	В	28.7	42.6	67%	В
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	D	27.3	42.4	65%	C	24.9	41.2	60%	C
	S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	20.2	42.5	47%	D	18.9	41.3	46%	D
12	S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	D	20.6	42.1	49%	D	15.9	39.1	41%	D
	S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	D	24.8	42.0	59%	C	21.6	39.0	55%	C
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	24.5	41.9	58%	С	18.2	41.8	43%	D
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	20.1	41.8	48%	D	16.9	41.8	40%	D
14	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	35.5	43.0	83%	В	31.1	43.0	72%	В
	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	31.0	43.2	72%	В	23.4	43.2	54%	C
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	D	13.5	32.1	42%	D	9.1	32.1	28%	F F
13	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	D	17.0	31.1	55%	C	15.3	31.1	49%	D
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	D	10.2	37.7	27%	F	13.2	37.7	35%	E
10	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	D	11.7	37.4	31%	E	32.0	37.7 37.4	85%	A
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	D	20.9	39.2	53%	C	17.9	39.2	46%	
17	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	D	20.9	39.2	75%	В	25.8	39.2	66%	C
18	Prado Rd		US 101 NB Ramps	WB	D	29.5	38.3	73%	В	22.2	38.3	58%	C
тя		S. Higuera St			D								-
10	Prado Rd	US 101 NB Ramps	S. Higuera St Los Osos Vallev	EB WB	D D	23.8 17.9	38.3 37.7	62% 47%	C D	21.2 12.7	38.3 37.9	55% 33%	<u>С</u> Е
19	Froom Ranch Way	Dick's Sporting Goods Drwy			D D	_							
20	Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods Drwy	EB	D	35.5 21.5	38.1 31.2	93% 69%	A	34.6	37.4 31.2	93% 69%	<u>А</u> В
20	Dalidio Dr	Madonna Rd	Froom Ranch Rd	SB					В	21.5			
	Dalidio Dr	Froom Ranch Rd	Madonna Rd	NB	D	5.5	31.1	18%	F	0.4	31.1	1%	F

TABLE 21: EXISTING CONDITIONS SEGMENT LEVEL OF SERVICE: PEDESTRIAN ANALYSIS

EDE	STRIAN SEGMENT LO	S					AM PI	EAK	PM PE	AK
					LOS	Average Ped.	SEGMENT		SEGMENT	
ID	Roadway	From	То	Direction	Threshold	Space (ft²/p)	SCORE	LOS	SCORE	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	С	6090	3.54	D	3.80	D
	Madonna Rd	LOVR	Oceanaire Dr	EB	С	17482	3.75	D	3.87	D
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	С	84000	3.62	D	3.92	D
	Madonna Rd	Oceanaire Dr	Dalidio	EB	С	26250	3.80	D	3.89	D
3	Madonna Rd	El Mercado	Dalidio Dr	WB	С	37450	3.52	D	3.78	D
	Madonna Rd	Dalidio Dr	El Mercado	EB	С	52920	3.63	D	3.73	D
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	С	26250	3.59	D	3.75	D
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	С	27915	3.84	D	4.04	D
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	С	No Peds	3.66	D	3.81	F
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	С	No Peds	4.06	D	3.98	D
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	С	25200	3.58	D	3.72	D
	Madonna Rd	US 101 NB Ramps	Higuera St	EB	С	19838	3.84	D	3.72	D
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	С	23247	3.80	D	3.78	D
	S. Higuera St	Margarita Ave	Madonna Rd	NB	С	5398	3.60	D	3.78	D
8	S. Higuera St	Margarita Ave	Prado Rd	SB	С	40979	3.61	D	3.63	D
	S. Higuera St	Prado Rd	Margarita Ave	NB	С	21700	3.47	С	3.57	D
9	S. Higuera St	Prado Rd	Granada Dr	SB	С	9292	3.55	D	3.65	D
	S. Higuera St	Granada Dr	Prado Rd	NB	С	8400	3.16	С	3.38	С
10	S. Higuera St	Granada Dr	Tank Farm Road	SB	С	46305	3.54	D	3.69	D
	S. Higuera St	Tank Farm Road	Granada Dr	NB	С	49140	3.11	С	3.26	С
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	С	12600	3.57	D	3.80	D
	S. Higuera St	Suburban Drive	Tank Farm Road	NB	С	31500	3.48	С	3.44	С
12	S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	С	39312	3.56	D	3.85	D
	S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	С	43533	3.84	D	3.89	D
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	С	21833	3.81	D	3.88	D
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	С	0	3.72	F	4.04	F
14	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	С	27300	3.76	D	3.97	D
	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	С	22050	3.67	D	3.94	D
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	С	No Peds	3.59	D	3.92	D
	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	c	63000	3.62	D	3.91	D
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	С	No Peds	4.15	D	4.19	D
	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	c	53928	3.62	D	3.85	D
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	C	46575	4.22	D	4.01	D
	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	С	1680	3.67	D	4.09	D
18	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	С	56133	2.90	С	3.10	С
	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	С	3019	3.42	С	3.32	С
19	Froom Ranch Way	Dick's Sporting Goods Drwy	Los Osos Valley	WB	С	No Peds	3.29	C	3.52	D
	Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods Drwy	EB	С	75600	1.67	Α	1.79	Α
20	Dalidio Dr	Madonna Rd	Froom Ranch Rd	SB	С	56700	1.46	Α	1.56	А
	Dalidio Dr	Froom Ranch Rd	Madonna Rd	NB	c	73710	3.04	С	3.35	С

Notes:

Sidewalk is present along frontage roads for segments #1 - Madonna Road and #13 - Los Osos Valley Road, and is not accounted for in this analysis.

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. No methodology exists for evaluating two-way stop-controlled interesction performance (with the cross-street stop controlled) for pedestrians and bicycles. However, it is reasoned that it has negligible influence on pedestrian sevice along the segment.

TABLE 22: EXISTING CONDITIONS SEGMENT LEVEL OF SERVICE: BICYCLE ANALYSIS

BIC	CYCLE SEGMENT LOS					AM PE	AK	PM PEAK	
ID	Roadway	From	То	Direction	LOS Threshold	SEGMENT SCORE	LOS	SEGMENT SCORE	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	D	3.60	D	3.94	D
	Madonna Rd	LOVR	Oceanaire Dr	EB	D	3.73	D	3.79	D
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	D	3.15	С	3.23	С
	Madonna Rd	Oceanaire Dr	Dalidio	EB	D	3.57	D	3.43	С
3	Madonna Rd	El Mercado	Dalidio Dr	WB	D	3.27	С	3.19	С
	Madonna Rd	Dalidio Dr	El Mercado	EB	D	3.49	С	3.39	С
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	3.94	D	4.34	Е
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	3.62	D	3.64	D
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	3.30	С	3.35	С
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	3.38	С	3.33	С
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	3.48	С	3.54	D
	Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	3.66	D	3.53	D
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	D	3.88	D	3.87	D
	S. Higuera St	Margarita Ave	Madonna Rd	NB	D	4.05	D	4.14	D
8	S. Higuera St	Margarita Ave	Prado Rd	SB	D	3.64	D	3.64	D
	S. Higuera St	Prado Rd	Margarita Ave	NB	D	3.87	D	3.92	D
9	S. Higuera St	Prado Rd	Granada Dr	SB	D	3.84	D	3.87	D
	S. Higuera St	Granada Dr	Prado Rd	NB	D	3.42	С	3.49	С
10	S. Higuera St	Granada Dr	Tank Farm Road	SB	D	4.10	D	4.16	D
	S. Higuera St	Tank Farm Road	Granada Dr	NB	D	3.48	С	3.52	D
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	D	3.33	С	3.41	С
	S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	3.40	С	3.39	С
12	S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	D	3.24	С	3.59	D
	S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	D	3.90	D	3.87	D
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	3.71	D	3.71	D
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	3.39	С	3.46	С
14	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	3.56	D	3.58	D
	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	3.80	D	3.88	D
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	D	3.33	С	3.52	D
	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	D	3.54	D	3.60	D
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	D	3.75	D	3.77	D
	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	D	3.61	D	3.50	С
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	D	3.40	C	3.40	c
	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	D	3.30	c	3.45	c
18	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	D	3.44	C	3.50	C
_5	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	D	3.94	D	3.68	D
19	Froom Ranch Way	Dick's Sporting Goods Drwy	Los Osos Valley	WB	D	3.29	C	3.47	
	Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods I	EB	D	2.87	c	3.48	Ċ
20	Dalidio Dr	Madonna Rd	Froom Ranch Rd	SB	D	4.30	E	4.37	E
	Dalidio Dr	Froom Ranch Rd	Madonna Rd	NB	D	3.77	D	4.21	D

Notes:

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. No methodology exists for evaluating two-way stop-controlled interesction performance (with the cross-street stop controlled) for pedestrians and bicycles. However, it is incorporated into the methodology for evaluateing bicycle segment performance.

TABLE 23: EXISTING CONDITIONS SEGMENT LEVEL OF SERVICE: TRANSIT ANALYSIS

TRAN	SIT SEGMENT LOS						AM PE	AK	PM PEAK	
					LOS	Route Name	SEGMENT		SEGMENT	
ID	Roadway	From	То	Direction	Threshold	Route Name	SCORE	LOS	SCORE	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	D	Ruote 4	4.16	D	4.27	E
	Madonna Rd	LOVR	Oceanaire Dr	EB	D	Route 5	4.28	E	4.10	D
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	D	Route 4	4.47	E	4.58	E
	Madonna Rd	Oceanaire Dr	Dalidio	EB	D	Route 5	4.70	E	4.51	E
3	Madonna Rd	El Mercado	Dalidio Dr	WB	D	Route 4	4.24	D	4.42	E
	Madonna Rd	Dalidio Dr	El Mercado	EB	D	Route 5	Not Analyzed	N/A	Not Analyzed	N/A
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	Route 4	4.31	E	4.53	E
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	Route 5	4.64	E	4.40	E
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	Route 4	3.89	D	3.99	D
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	Route 5	4.29	E	3.89	D
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	Route 4	4.27	E	4.37	Е
	Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	Route 5	4.50	E	4.18	D
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	D	Route 2	Not Analyzed	N/A	3.49	С
	S. Higuera St	Margarita Ave	Madonna Rd	NB	D	Route 2	3.65	D	3.67	D
8	S. Higuera St	Margarita Ave	Prado Rd	SB	D	Route 2	Not Analyzed	N/A	4.19	D
	S. Higuera St	Prado Rd	Margarita Ave	NB	D	Route 2	4.15	D	4.24	D
9	S. Higuera St	Prado Rd	Granada Dr	SB	D	Route 2	4.35	E	4.28	E
,	S. Higuera St	Granada Dr	Prado Rd	NB	D	Route 2	3.76	D	3.82	D
10	S. Higuera St	Granada Dr	Tank Farm Road	SB	D	Route 2	3.75	D	3.83	D
	S. Higuera St	Tank Farm Road	Granada Dr	NB	D	Route 2	3.51	D	3.58	D
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
	S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	Route 2	3.97	D	3.96	, D
12	S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
	S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	Route 4	4.49	E	4.53	E
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 4	4.23	D	4.38	E
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	Route 5	4.59	E	4.38	E
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 5	4.37	E	4.21	D
14	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	Route 4	4.21	D	4.33	E
	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	Route 4	4.21	D	4.44	E
14	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	Route 5	4.37	E	4.15	D
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 5	4.35	E	4.28	E
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	D		Not Analyzed	N/A	Not Analyzed	N/A
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	D		Not Analyzed	N/A	Not Analyzed	N/A
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	D		Not Analyzed	N/A	Not Analyzed	N/A
4.0	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
18	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	D	Route 2	3.83	D	Not Analyzed	N/A
19	Froom Ranch Way	Dick's Sporting Goods Drwy	Los Osos Valley	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods Drwy	EB	D		Not Analyzed	N/A	Not Analyzed	N/A
20	Dalidio Dr	Madonna Rd	Froom Ranch Rd	SB	D	Route 4	4.01	D	4.10	D
	Dalidio Dr	Madonna Rd	Froom Ranch Rd	SB	D	Route 5	4.26	E	4.09	D

Note: Route 2 Serves the Prado Day Center stop during the AM peak hour, and the DMV/Margarita stop during the PM Peak Hour

Segment 20 transit is southbound for routes 4 and 5 $\,$

TABLE 24: EXISTING CONDITIONS SEGMENT LEVEL OF SERVICE: FREEWAY ANALYSIS

				AM Peak Hour			PM Peak Hour		
	Target	Segment	No. of		Density			Density	
Interchange Location	LOS	Type	Lanes	Volume	(pc/mi/ln)	LOS	Volume	(pc/mi/ln)	LOS
US 101 at Los Osos Valley Road									
US 101 NB South of Los Osos Valley Road	С	Freeway	2	2,774	24.5	O	2,249	19.7	С
US 101 SB South of Los Osos Valley Road	С	Freeway	2	1,406	12.3	В	3,044	27.4	D
US 101 at Prado Road				•					
US 101 NB South of Prado Road	С	Freeway	2	2,443	21.4	С	2,137	18.8	С
US 101 at Madonna Road				•					
US 101 NB South of Madonna Road	С	Freeway	2	2,468	21.7	С	2,497	21.9	С
US 101 SB South of Madonna Road	С	Freeway	2	1,663	14.6	В	2,881	25.6	С

As shown in the Existing Conditions segment analysis tables, most of the segments are currently operating at deficient LOS for pedestrian and transit modes, as well as several segments for automobile mode and only a couple segments for the bicycle mode. It is important to note that segment pedestrian and transit LOS, which are strongly interrelated, are heavily influenced by vehicular volumes, such that in some cases, LOS may not be reasonably improved by improved pedestrian facilities. Rather, a reduction in traffic volumes is often the only way to theoretically achieve exceptional LOS scores for these modes. The segment of US 101 south of Los Osos Valley Road is operating at deficient LOS D during the PM peak hour for Existing conditions.

Proposed Project Description

The proposed project is a mixed-use development located in between Madonna Road and Highway 101 on a 131-acre infill site, known as the San Luis Ranch Specific Plan and previously known also as Dalidio Ranch. The site is currently in the City's Sphere of Influence but would be annexed with Specific Plan adoption. The project would incorporate a mix of residential and commercial uses, as described in the 580-Unit Alternative of the San Luis Ranch Specific Plan. The proposed Specific Plan would maintain and promote San Luis Obispo's agricultural heritage, provide open space and recreation areas, provide diverse housing, and create a multimodal community that is integrated into the existing circulation system.

The project would contain the following:

- Single Family Residential (large-lot): 200 Dwelling Units
- Single Family Residential (small-lot, medium density): 100 Dwelling Units
- Multi-Family Residential (high-density, including Affordable Units): 280 Dwelling Units
- Commercial: 150,000 Sq Ft
- Hotel: 200 RoomsOffice: 100,000 Sq Ft
- Parks & Recreational: 5.8 Acres (3.39 acres parks and 2.41 acres recreation for the Bob Jones Trail)
- Agricultural Heritage and Learning Center: 2.70 acres

Project Access

The proposed project would have various access points and roadway improvements upon project build-out. Froom Ranch Way would be extended as a two-lane collector with a Class I bike path to Dalidio Drive with a bridge over Perfumo Creek connecting to the current alignment. The proposed project would have full access to Froom Ranch Way at two locations. Dalidio Drive improvements include widening to four-lane arterial standards with Class II bike lanes with project build-out. The proposed project access on Dalidio Drive would be full-access and located across from the existing shopping center driveway approximately 900 feet south of Madonna Road. Madonna Road improvements with the build-out of the project include a right-in right-out project access point with acceleration and deceleration lanes to match the segments east and west of the Specific Plan. Project access for the proposed office uses to the east of Dalidio Drive is projected to be via the intersection of Froom Ranch Way and Dalidio Drive.

The project site plan is presented in Figure 5.

Access Management

The proposed project driveways and street connections to the existing network are evaluated using the recommendations and guidelines established in the Transportation Research Board Access Management Manual, 2nd Ed., 2014. In particular, intersection spacing, residential neighborhood traffic calming aspects and internal site circulation are evaluated. The evaluation is included in the Appendix of this report as a Memorandum. The following recommendations are included in the memorandum:

- Intersection #26: Dalidio Drive/Post Office Access/Promenade
 - Remove existing ingress only driveway to Post Office

- Consolidate the Post Office access to one limited access driveway
- The modified post office driveway is proposed to be limited to right turn ingress and egress and left turn ingress only
- The shopping center driveway would be limited to right turn ingress and egress only
- Include a raised crosswalk where the proposed bike/pedestrian crossing south of curve number 9 (Bike Path connection between Multi-Family and Single-Family areas)
- Install three neighborhood traffic circles (as shown on Figure 4 of Memorandum) to reduce speeds in curve numbers 1 and 8 and in the long tangent section between Froom Ranch Way and curve number 9.

Neighborhood Traffic Management

San Luis Ranch Road would provide a bypass to the Madonna and Dalidio/Prado Rd. intersection thru the residential neighborhood resulting in a high propensity for cut through traffic and volumes that exceed general plan thresholds. The addition of diverters or changes in street connections would reduce this to a less than significant impact.

Design of San Luis Ranch Road, XXXXX Road, XXXXX Road, & XXXXX Road would result in a high propensity of neighborhood traffic speeds exceeding general plan thresholds. The additional of neighborhood traffic circles at key intersections and other traffic calming features along longer uninterrupted segments would reduce this to a less than significant impact.



Multimodal Trip Generation

Since this study analyzes and quantifies impacts for all travel modes rather than just automobile impacts, a more defined "person trip" generation was developed in lieu of the more common "vehicle trip" generation. The following section describes how the "person trips" estimates were developed for the proposed land uses, with respect to the following categories:

- Vehicular Trips
- Person Trips
- Internal Capture
- Modal Split
- Vehicle Occupancy
- Pass By Trips

Vehicular Trips

Vehicular trips were estimated using the Institute of Transportation Engineers (ITE) *Trip Generation Manual (9th Edition)*. The "Shopping Center" Land Use was used to determine the trip generation for the commercial element during the AM and PM peak hour. The *ITE Trip Generation Manual* does not contain adequate information for the Parks/Recreational land use. Omni-Means used San Diego's Regional Planning Agency's (SANDAG) '*Brief Guide to Vehicular Traffic Generation Rates for the San Diego Region*', 2002 to determine the trip generation for the Parks/Recreational land use using the City Park land use generation rate as the best available rate which is expected to have similar trip generation characteristics. Table 25 presents the ITE trip generation rates and estimates for the proposed land uses.

TABLE 25: VEHICULAR TRIP GENERATION RATES AND ESTIMATES

		Daily Trip	AM Deals Hassa Take Date / lock			PM Peak	Hour Trip	Rate/Unit
Land Use Category (ITE Code)	Unit ¹	ruto/onit	Total	In %	Out %	Total	In %	Out %
Single Family Detached (210)	DU	9.62	0.73	25%	75%	0.93	63%	37%
Apartment (220)	DU	6.50	0.50	20%	80%	0.61	65%	35%
Shopping Center (820)	KSF	58.93	1.33	62%	38%	5.24	48%	52%
Hotel (310)	Rooms	8.17	0.53	59%	41%	0.60	51%	49%
General Office Building (710)	KSF	13.13	1.91	88%	12%	1.90	17%	83%
City Park ³	AC	50.00	6.50	50%	50%	4.50	50%	50%
City Park ³	AC	50.00	6.50	50%	50%	4.50	50%	50%
	Quantity	Doily Tring	AM Peak Hour Trips		PM F	PM Peak Hour Trips		
Project Name	(Units)	Daily Trips	Total	ln	Out	Total	ln	Out
Single Family Residential (Small 30' lots)4	100	962	73	18	55	93	59	34
Single Family Residential (Traditional 40' lots)	200	1,924	147	37	110	185	117	68
Multi-Family Residential	280	1,820	141	28	113	172	112	60
Commercial	150	8,839	200	124	76	786	377	409
Hotel	200	1,634	106	63	43	120	61	59
Office	100	1,313	191	168	23	190	32	158
Parks/Recreational	5.80	290	38	19	19	26	13	13
Agriculture Heritage and Learning Center	2.70	135	18	9	9	12	6	6
Net Project Trips		16,917	914	466	448	1,584	777	807

Notes:

^{1. 1} ksf = 1,000 square feet DU = dwelling unit

^{2.} Trip rates based on ITE Trip Generation Manual 9th edition fitted-curve equations or average rates

^{3.}Trip rates baed on SANDAG's 'Brief Guide to Vehicular Traffic Generation Rates for the San Diego Region', 2002

^{4.} Single Family Residential Units Trip Generation Based on Rate for Total 350 Units

^{5.} Errors due to rouding may occur.

As presented in Table 25, the proposed project would be projected to generate 914 AM peak hour vehicular trips, and 1,559 PM peak hour vehicular trips, before any vehicular occupancy, modal, internal capture, or pass-by adjustments. Once person trips were estimated, internal capture rates were developed between the proposed uses.

Person Trips

Person trips were also estimated using the ITE *Trip Generation Manual*, by utilizing average vehicle occupancy rates of each Land Use (Tables C.1-C.2). Because ITE *Trip General Manual* vehicle trip generation rates (Table 25) are based on driveway counts, an additional factor called the "Mode Share Percent" is also utilized to account for trips that were made by other modes. After applying vehicle occupancy rates to the vehicle trip rates, and then accounting for demand from non-auto modes in the ITE data, an ITE-based person trip demand can be estimated, as shown in Table 26.

The ITE *Trip Generation Handbook (3rd Edition)* provides the Land Use Mix for determining whether the internal capture methodology is appropriate, stating that the mixed-use development should consist of a combination of at least two of the following categories: retail, restaurant, office, residential, hotel, and cinema/entertainment. The proposed project contains retail, residential, office and hotel land uses, which are generated separately for each category, and therefore accounts for the internal trips between each land use.

Internal Capture

Internal person trip capture rates for the proposed development were determined using the NCHRP Report 864 and NCHRP 8-51 Internal Capture Estimation Tool, which is consistent with the internal capture percentages from the ITE *Trip Generation Handbook (3rd Edition)*. The NCHRP Internal Capture Estimation Tool also includes a proximity adjustment factor to the internal capture percentages for the PM peak hour. For the purposes of this study, the internal capture reduction for residential uses included only the multi-family units and the single family small-lot (30' lot) units due to their proximity to other proposed uses. The single-family large-lot (40' lot) units were not assumed to generate internal trips. Table 26 presents the person trip generation and the internal capture reduction between each use.

As presented in Table 26, after applying internal capture assumption to the estimated person trips, the proposed project is anticipated to generate 1,009 AM peak hour person trips and 1,538 PM peak hour person trips. The internal capture reduction is approximately 8% in the AM peak hour and 20% in the PM peak hour.

TABLE 26: PERSON TRIP GENERATION RATES AND ESTIMATES

Assumption	ons to Conven	t Baseline Veh	nicular Trips	to Person Ti	rips:		
			AM Peak			PM Peak	
		Baseline Vehicle		lode Share cent ²	Baseline Vehicle		lode Share cent ²
Land Use	Unit ¹	Occupancy ²	Inbound	Outbound	Occupancy ²	Inbound	Outbound
Single Family Detached (210)	DU	1.11	95.0%	95.0%	1.18	95.0%	95.0%
Single Family Detached (210)	DU	1.11	95.0%	95.0%	1.18	95.0%	95.0%
Apartment (220)	DU	1.11	89.2%	96.8%	1.18	96.3%	94.7%
Shopping Center (820)	KSF	1.17	100.0%	100.0%	1.20	100%	99.8%
Hotel (310)	Rooms	1.26	92.3%	96.7%	1.31	95.7%	95.0%
General Office Building (710)	KSF	1.06	97.0%	90.0%	1.09	96.0%	98.0%
City Park ⁴	AC	1.17	95.0%	95.0%	1.20	95.0%	95.0%
City Park ⁴	AC	1.17	95.0%	95.0%	1.20	95.0%	95.0%

	Quantity	AM Peal	k Hour Perso	n Trips	PM Peak	Hour Persor	Trips
Land Use	(Units)	Total	In	Out	Total	In	Out
Single Family Residential (Small 30' lots)	100	85	21	64	116	73	43
Single Family Residential (Traditional 40' lots)	200	172	43	129	229	145	84
Multi-Family Residential	280	165	35	130	212	137	75
Internal Capture Reduction	on with Retail⁵	-3	-1	-2	-126	-97	-29
Internal Capture Reducti	on with Hotel ⁵	0	0	0	-4	0	-4
Internal Capture Reduction	on with Office ⁵	-4	0	-4	-7	-2	-5
TOTAL RESIDENTIAL EXTERNAL TRI	IPS	415	98	317	420	256	164
Commercial	150	234	145	89	944	452	492
Internal Capture Reduction with	h Residential⁵	-3	-2	-1	-126	-29	-97
Internal Capture Reducti	on with Hotel⁵	-6	-6	0	-23	-9	-14
Internal Capture Reduction	on with Office ⁵	-15	-8	-7	-31	-21	-10
TOTAL COMMERCIAL EXTERNAL TR	IPS	210	129	81	764	393	371
Hotel	200	142	86	56	165	84	81
Internal Capture Reduction with	h Residential⁵	0	0	0	-4	-4	0
Internal Capture Reduction	on with Retail⁵	-6	0	-6	-23	-14	-9
Internal Capture Reduction	on with Office ⁵	-6	0	-6	0	0	0
TOTAL HOTEL EXTERNAL TRIPS		130	86	44	138	66	72
Office	100	211	184	27	212	36	176
Internal Capture Reduction with	h Residential⁵	-4	-4	0	-7	-5	-2
Internal Capture Reduction	on with Retail⁵	-15	-7	-8	-31	-10	-21
Internal Capture Reducti	on with Hotel ⁵	-6	-6	0	0	0	0
TOTAL OFFICE EXTERNAL TRIPS		186	167	19	174	In 73 145 137 -97 0 -2 256 452 -29 -9 -21 393 84 -4 -14 0 66 36 -5 -10	153
Parks/Recreational	5.80	46	23	23	32	16	16
Agriculture Heritage and Learning Center	2.70	22	11	11	16	8	8
TOTAL PARKS/REC/AG EXTERNAL TR	RIPS	68	34	34	48	24	24
Total Person Trips		1,077	548	529	1,926	951	975
Total Internal Capture Reduction)	-68	-34	-34	-382	-191	-191
TOTAL EXTERNAL PERSON TRIPS	3	1,009	514	495	1,544	760	784

^{1. 1} ksf = 1,000 square feet DU = dwelling unit

^{2.} ITE Trip Generation Handbook (3rd Ed.) Tables C.1 and C.2

^{3.} Vehicle Occupancy is determined to be the same as Apartment, mode share is 95% vehicular as baseline default, due to unavailable data within the ITE Trip Generation Handbook

^{4.} Vehicle Occupancy is determined to be the same as Shopping Center, mode share is 95% vehicular as baseline default, due to unavailable data within the ITE Trip Generation Handbook

^{5.} Internal Capture Trips Determined using NCHRP Report 864 and NCHRP 8-51 Internal Capture Estimation Tool, which includes a proximity adjustment for the PM peak hour; Residential Reduction based only on Small Lots and Multi-Family Units due to proximity to other uses

^{6.} Rounding errors may occur.

Modal Split

The transportation network adjacent to the project site consists of auto, transit, bicycles, and pedestrian modes. It is anticipated that new external person trips to the proposed project would use a variety of transportation modes to get to and from the site. To estimate the travel mode splits for the external person trips, data from the City of San Luis Obispo Travel Demand Model (TDM) and the US Census were used. The City's TDM contains transit data for each route, including transit route volumes, which were used to determine the percent of person-trips using walk-accessible transit. The vehicular, pedestrian, and bicycle volumes were collected from the City's most recent roadway counts. The parks and recreational uses were determined to be split between bicycles and pedestrians. The mode split among the project-generated person trips is projected to be consistent for Existing and Near Term conditions. The mode split for the Cumulative Year 2035 conditions is based on forecasted modal shifts within the City's TDM at buildout of the City General Plan. Table 27A shows the mode split percentage summary for the external person trips for the Existing and Near Term assignments.

TABLE 27A: PERSON TRIPS MODE SPLIT FOR EXISTING AND NEAR TERM ASSIGNMENTS

		AM Peal	k Hour Pers	on Trips	PM Peal	· Hour Pers	on Trips
Land	Use	Total	In	Out	Total	In	Out
TOTAL RESIDENTIAL	EXTERNAL TRIPS	415	98	317	420	256	164
TOTAL COMMERCIA	L EXTERNAL TRIPS	210	129	81	764	393	371
TOTAL HOTEL EX	(TERNAL TRIPS	130	86	44	138	66	72
TOTAL OFFICE EX	KTERNAL TRIPS	186	167	19	174	21	153
TOTAL PARKS/REC	EXTERNAL TRIPS	46	23	23	32	16	16
TOTAL AGRICULTURA	AL EXTERNAL TRIPS	22	11	11	16	8	8
TOTAL EXTERNAL	PERSON TRIPS	1,009	514	495	1,544	760	784
Total External Person	Trips for Residential,	963	491	472	1,512	744	768
M	ODE SPLIT - PERSON	TRIPS (fo	r Non-Park	s/Recreation	onal Uses)		
		AM Peal	k Hour Pers	on Trips	PM Peal	Hour Pers	on Trips
Mode	Modal Split %1	Total	In	Out	Total	In	Out
Vehicle	97.0%	934	476	458	1,467	722	745
Bicycle	1.1%	10	5	5	16	8	8
Pedestrian	0.5%	4	2	2	8	4	4
Transit	1.4%	14	7	7	21	10	11
Total:	100%	962	490	472	1,512	744	768
Total External P	erson Trips for	46	23	23	32	16	16
	MODE SPLIT - PERSO	ON TRIPS	for Parks/l	Recreation	al Uses)		
		AM Peal	Hour Pers	on Trips	PM Peal	Hour Pers	on Trips
Mode	Modal Split %	Total	In	Out	Total	In	Out
Vehicle	0%	0	0	0	0	0	0
Bicycle	50%	24	12	12	16	8	8
Pedestrian	50%	24	12	12	16	8	8
Transit	0%	0	0	0	0	0	0
Total:	100%	48	24	24	32	16	16

^{1.} Mode Split Determined using existing counts for Non-Parks/Recreational Uses, and assumed for Parks/Recreational Uses due to connectivity to existing mulitmodal transporation system

The net new person trips (auto, pedestrian, bicycle, and transit) are trips that are added to the transportation network. In the AM peak hour, it is projected that 14 transit trips, 28 pedestrian and 34 bicycle trips would be added to the network. In the PM peak hour, it is projected that 20

^{2.} Person Trip estimates for each mode are rounded to the nearest integer.

transit, 24 pedestrian, and 32 bicycle trips would be added to the network with the proposed project. It is also projected that 934 vehicle "person trips" in the AM peak hour and 1,461 vehicle "person trips" in the PM peak hour are added to and from the external roadway network.

Table 27B shows the mode split percentage summary for the external person trips for the 2035 Cumulative conditions.

TABLE 27B: PERSON TRIPS MODE SPLIT FOR CUMULATIVE ASSIGNMENTS

		AM Peal	k Hour Pers	on Trips	PM Peal	k Hour Pers	on Trips
La	nd Use	Total	ln	Out	Total	In	Out
TOTAL RESIDENT	TAL EXTERNAL TRIPS	415	98	317	420	256	164
TOTAL COMMERC	CIAL EXTERNAL TRIPS	210	129	81	764	393	371
TOTAL HOTEL	. EXTERNAL TRIPS	130	86	44	138	66	72
TOTAL OFFICE	EXTERNAL TRIPS	186	167	19	174	21	153
TOTAL PARKS/R	EC EXTERNAL TRIPS	46	23	23	32	16	16
TOTAL AGRICULTU	JRAL EXTERNAL TRIPS	22	11	11	16	8	8
TOTAL EXTER	NAL PERSON TRIPS	1,009	514	495	1,544	760	784
Total External Pers	on Trips for Residential,	963	491	472	1,512	744	768
	MODE SPLIT - PERSON	I TRIPS (fo	r Non-Park	s/Recreation	onal Uses)		
		AM Peal	k Hour Pers	on Trips	PM Peal	k Hour Pers	on Trips
Mode	Modal Split %1	Total	In	Out	Total	In	Out
Vehicle	94.9%	914	466	448	1,435	706	729
Bicycle	1.1%	10	5	5	16	8	8
Pedestrian	2.7%	26	13	13	40	20	20
Transit	1.3%	12	6	6	20	10	10
Total:	100%	962	490	472	1,511	744	767
Total Externa	l Person Trips for	46	23	23	32	16	16
	MODE SPLIT - PERS	ON TRIPS	(for Parks/	Recreation	al Uses)		
		AM Peal	k Hour Pers	on Trips	PM Peal	Hour Pers	on Trips
Mode	Modal Split %	Total	In	Out	Total	In	Out
Vehicle	0%	0	0	0	0	0	0
Bicycle	50%	24	12	12	16	8	8
Pedestrian	50%	24	12	12	16	8	8
Transit	0%	0	0	0	0	0	0
Total:	100%	48	24	24	32	16	16

^{1.} Mode Split Determined using existing counts for Non-Parks/Recreational Uses, and assumed for Parks/Recreational Uses due to connectivity to existing mulitmodal transporation system

The net person trips (auto, pedestrian, bicycle, and transit) are trips that are added to the transportation network. In the AM peak hour, it is projected that 14 transit trips, 48 pedestrian and 34 bicycle trips are added to the network. In the PM peak hour, it is projected that 21 transit, 24 pedestrian, and 32 bicycle trips are added to the network with the proposed project. It is also projected that 934 vehicle "person trips" in the AM peak hour and 1,461 vehicle "person trips" in the PM peak hour are added to and from the external roadway network.

Vehicle Occupancy

Vehicular occupancy was determined using the data from the City's TDM Documentation. Table 28 shows the home-, work-, and other-based-other data from the *Number of Trip Observations* by Mode and Purpose from 2000-2001 Statewide Travel Survey for City of San Luis Obispo (Table 5.1 SLO City Travel Model Documentation).

^{2.} Person Trip estimates for each mode are rounded to the nearest integer.

TABLE 28: VEHICLE OCCUPANCY DETERMINATION

		Home-	Home-	Home-	Home-	Work-	Other-			
	Total of all	Based	Based	Based	Based	Based	Based			
Mode	purpose	Work	Shop	Universi	Other	Other	Other	Total	% of Total	Persons
Drove Alone	2,704	724	410	39	662	457	412	2,704	49%	2,704
Shared Ride 2	750	28	132	7	308	41	234	750	14%	1,500
Shared Ride 3	182	4	25	-	101	5	47	182	3%	546
Shared Ride 4	55	-	4	-	33	5	13	55	1%	220
Shared Ride 5	29	-	-	-	13	-	16	29	1%	145
Shared Ride 6	4	-	-	-	3	-	1	4	0%	24
Shared Ride 7	-	-	-	-	-	-	-	-	0%	-
Shared Ride 8	-	-	-	-	-	-	-	-	0%	-
Shared Ride 9	1	-	-	-	-	2	-	2	0%	18
Total Vehicle Trips	3,724	756	571	46	1,120	510	723	3,726	68%	5,157
Auto Passenger	1,312	59	178	8	675	36	356	1,312	24%	_
Bus	17	-	2	4	5	-	6	17	0.3%	
Bicycle	77	13	13	9	25	7	10	77	1%	
School Bus	109	-	-	2	82	-	25	109	2%	
Walk	236	23	10	2	104	33	64	236	4%	Veh. Occ
Total	5,475	851	774	71	2,011	586	1,184	5,477		1.38

As shown in Table 28, the vehicle occupancy rate was calculated as the sum of the product of persons per vehicle (1 - 9) and the number of total number of trips made by each mode type, divided by the total number of vehicle trips, which is 1.38; The Total Persons per Total Vehicle Trips for the City of San Luis Obispo was reported to be 1.38.

Pass By Trips

A pass-by percentage of 10% in the AM peak and 30% in the PM peak hour are used for the shopping center and applied to the net vehicular trips. Table 29A presents the vehicular trips for the generated vehicular external person trips and the net new vehicular trips for Existing and Near Term conditions. Table 29B presents the vehicular trips for the generated vehicular external person trips and the net new vehicular trips for Cumulative Year 2035 conditions.

As shown in Table 29A for Existing and Near Term conditions, a total of 662 vehicles in the AM peak hour and 899 vehicles in the PM peak hour are projected to be added to the roadway network due to the proposed project. As shown in Table 29B for Cumulative Year 2035 conditions, a total of 648 vehicles in the AM peak hour and 879 vehicles in the PM peak hour are projected to be added to the roadway network due to the proposed project.

TABLE 29A: EXTERNAL VEHICULAR TRIPS & PASS-BY TRIPS (EXISTING AND NEAR TERM CONDITIONS)

TOTAL EXTERNAL VEHICULAR PERSON TRIPS BY	AM Peak	Hour Pers	son Trips	PM Peak	Hour Pers	son Trips
LAND USE	Total	ln	Out	Total	ln	Out
RESIDENTIAL	403	95	308	408	248	159
COMMERCIAL	204	125	79	741	381	360
HOTEL	126	83	43	134	64	70
OFFICE	180	162	18	169	20	148
PARKS/RECREATIONAL	0	0	0	0	0	0
AGRICULTURAL CENTER	21	11	11	16	8	8
TOTAL EXTERNAL VEHICLE AR REPORT TRIPS		4=0	450	4 40=		
TOTAL EXTERNAL VEHICULAR-PERSON TRIPS	934	476	458	1,467	722	745
VEHICULAR OCCUPANCY ¹			1.38			

Vehicular Trips Conversion by Land Use

	Al	M Peak Ho	ur	PI	/IPeak Ho	ur
Land Use	Total	In	Out	Total	In	Out
Residential	292	69	223	295	180	115
Hotel	91	60	31	97	46	51
Office	130	117	13	123	15	108
Parks/Recreational	0	0	0	0	0	0
Agricultural	16	8	8	12	6	6
Commercial	148	91	57	537	276	261
Pass-by Trips for Shopping Center ²	-15	-9	-6	-161	-83	-78
Net New Commercial Vehicular Trips:	133	82	51	376	193	183
Net New Vehicular Trips:	662	336	326	903	440	463

^{1.} Vehicle Occupancy determined using Citywide Travel Demand Model Documentation, Table 5.1

TABLE 29B: EXTERNAL VEHICULAR TRIPS & PASS-BY TRIPS (CUMULATIVE CONDITIONS)

TOTAL EXTERNAL VEHICULAR PERSON TRIPS BY	AM Peak	Hour Pers	son Trips	PM Peak	Hour Pers	son Trips
LAND USE	Total	ln	Out	Total	In	Out
RESIDENTIAL	394	93	301	399	243	156
COMMERCIAL	199	122	77	725	373	352
HOTEL	123	82	42	131	63	68
OFFICE	177	158	18	165	20	145
PARKS/RECREATIONAL	0	0	0	0	0	0
AGRICULTURAL CENTER	21	10	10	15	8	8
	ı	· I		·	ı I	
TOTAL EXTERNAL VEHICULAR-PERSON TRIPS	914	466	448	1,435	706	729
VEHICULAR OCCUPANCY ¹			1.38			

Vehicular Trips Conversion by Land Use

	Al	M Peak Ho	ur	PI	VIPeak Ho	ur
Land Use	Total	In	Out	Total	In	Out
Residential	285	67	218	289	176	113
Hotel	89	59	30	95	45	50
Office	128	115	13	119	14	105
Parks/Recreational	0	0	0	0	0	0
Agricultural	16	8	8	12	6	6
Commercial	145	89	56	525	270	255
Pass-by Trips for Shopping Center ²	-15	-9	-6	-158	-81	-77
Net New Commercial Vehicular Trips:	130	80	50	367	189	178
Net New Vehicular Trips:	648	329	319	882	430	452

^{1.} Vehicle Occupancy determined using Citywide Travel Demand Model Documentation, Table 5.1

^{2.} Pass-By Trips based on ITE Trip Generation Handbook, 3rd Ed. For Non-Residential Uses; determined to be 10% for AM peak 30% for PM peak, for the Shopping Center

^{2.} Pass-By Trips based on ITE Trip Generation Handbook, 3rd Ed. For Non-Residential Uses; determined to be 10% for AM peak 30% for PM peak, for the Shopping Center

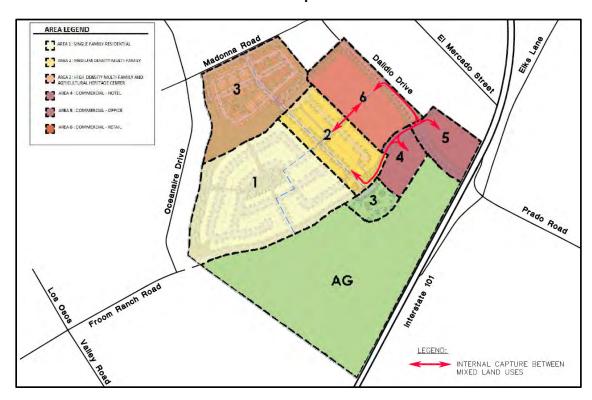
Existing Plus Project Conditions

The project generated peak hour trip volumes were added to the Existing volumes to obtain the Existing Plus Project conditions. The LOS has been quantified and compared to all study intersections and roadways analyzed in Existing (Base) conditions. All proposed driveway intersections have been evaluated for LOS operations, potential vehicle queuing and pedestrian and bicycle level of service. Impacts caused by the generation of new vehicle, pedestrian, bike, and auto trips were identified. Improvements to mitigate any impacts have also been identified.

Trip Distribution

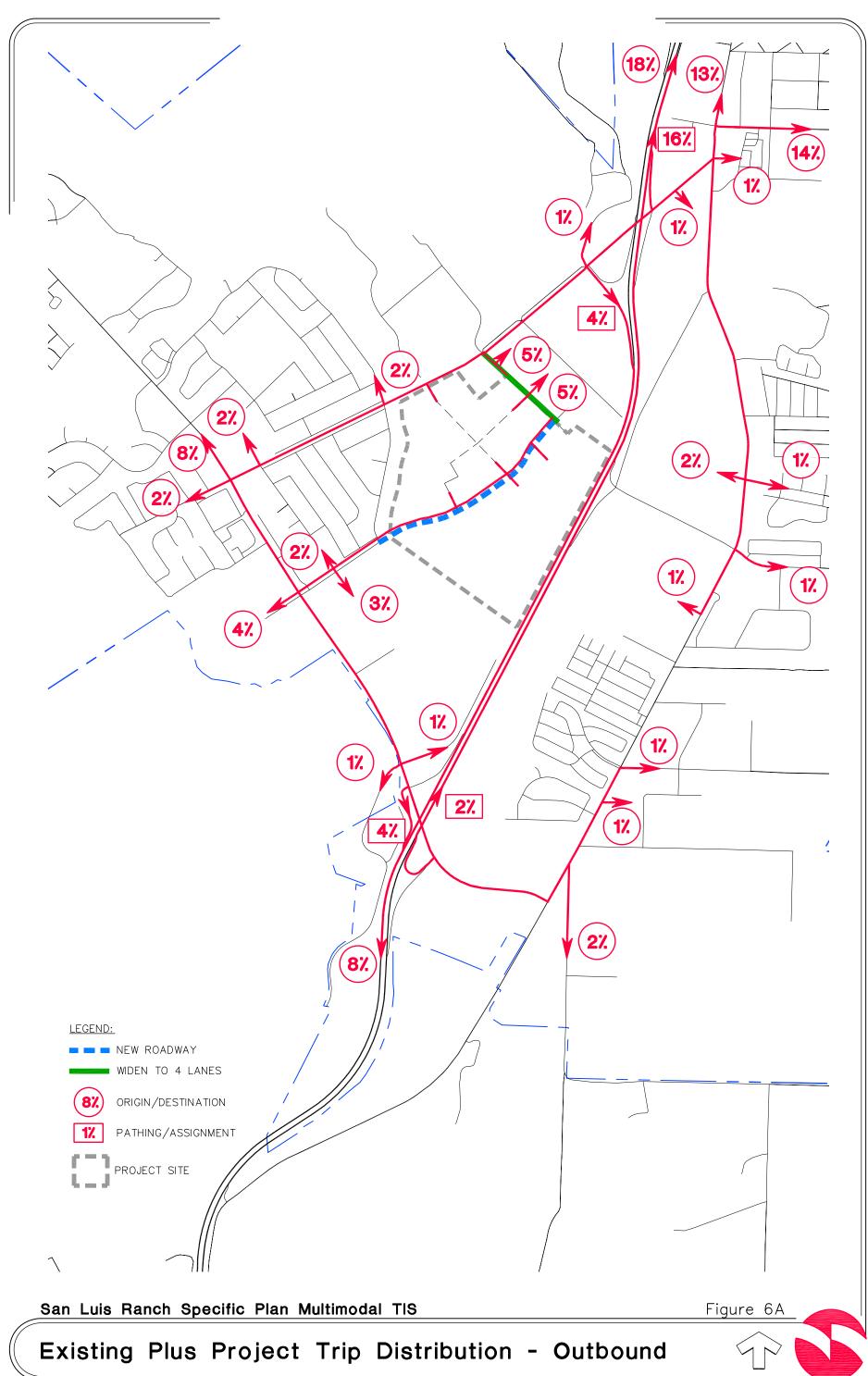
The Existing Plus Project distribution of project-generated pedestrian, bicycle, and vehicle trips has been estimated based on conducting a select zone analysis of the TAZ that represents the proposed project using the City's Travel Demand Model (TDM), and by comparing the existing traffic counts and associated travel patterns with the surrounding land uses. New transit users are assumed to all use the proposed new transit hub on Dalidio Drive. Bicyclists are assumed to use new and existing bike trails, Froom Ranch Way, Dalidio Drive, Prado Road, and Madonna Road. Pedestrians are assumed to use adjacent sidewalks, crosswalks, and shared-use paths enroute to/from the project based on a 1/2 – 3/4 mile walking radius between trip origins and destinations.

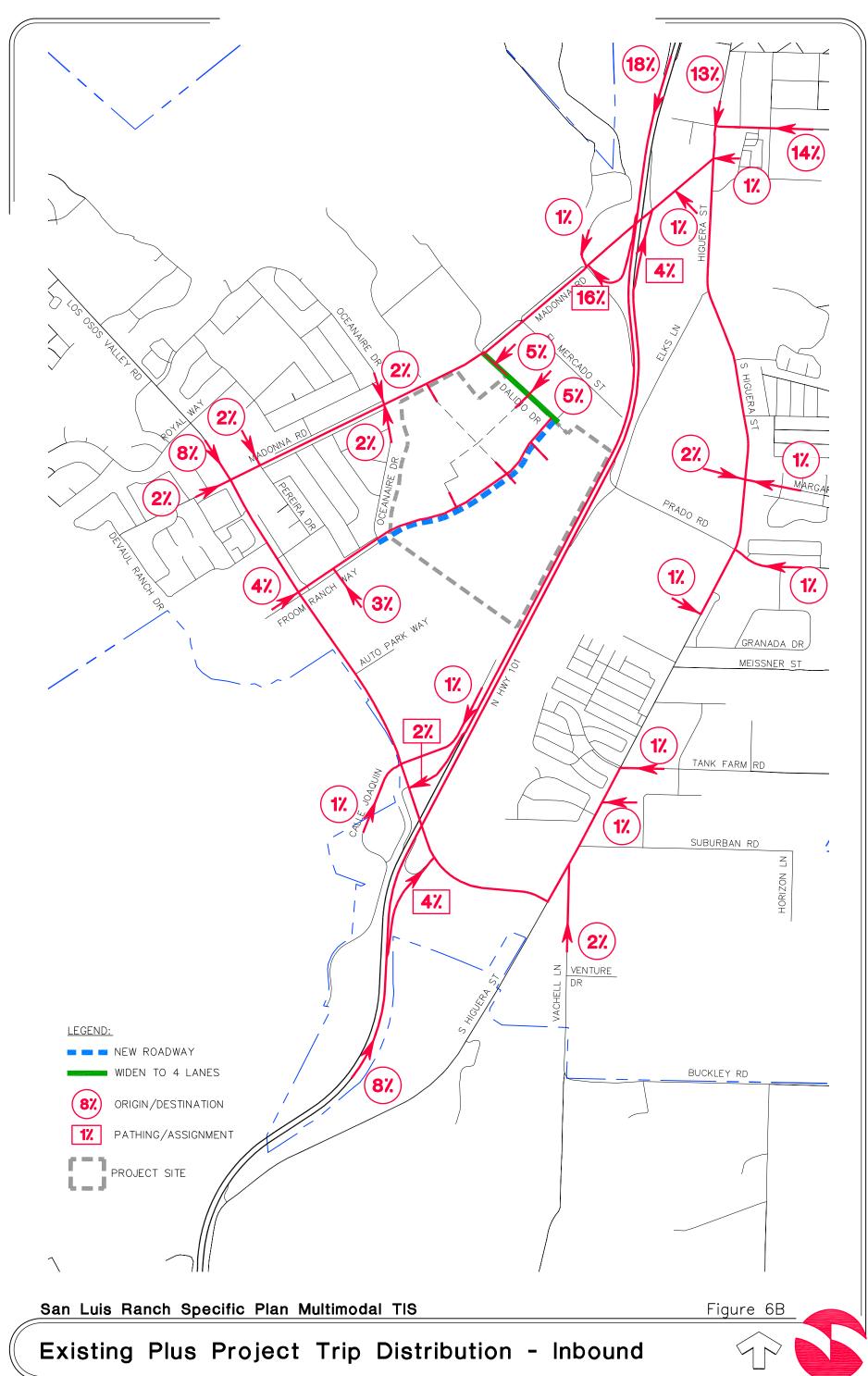
Since two of the study intersections are internal to the project site, the internally captured trips between proposed uses have been accounted for along the internal street system. The internal trip mode split is assumed to be 50% vehicular, 25% pedestrian, and 25% bicycle due to the large site plan, network layout and proximity of the uses. Inset A presents the internal capture distribution for the site's internal study intersections.

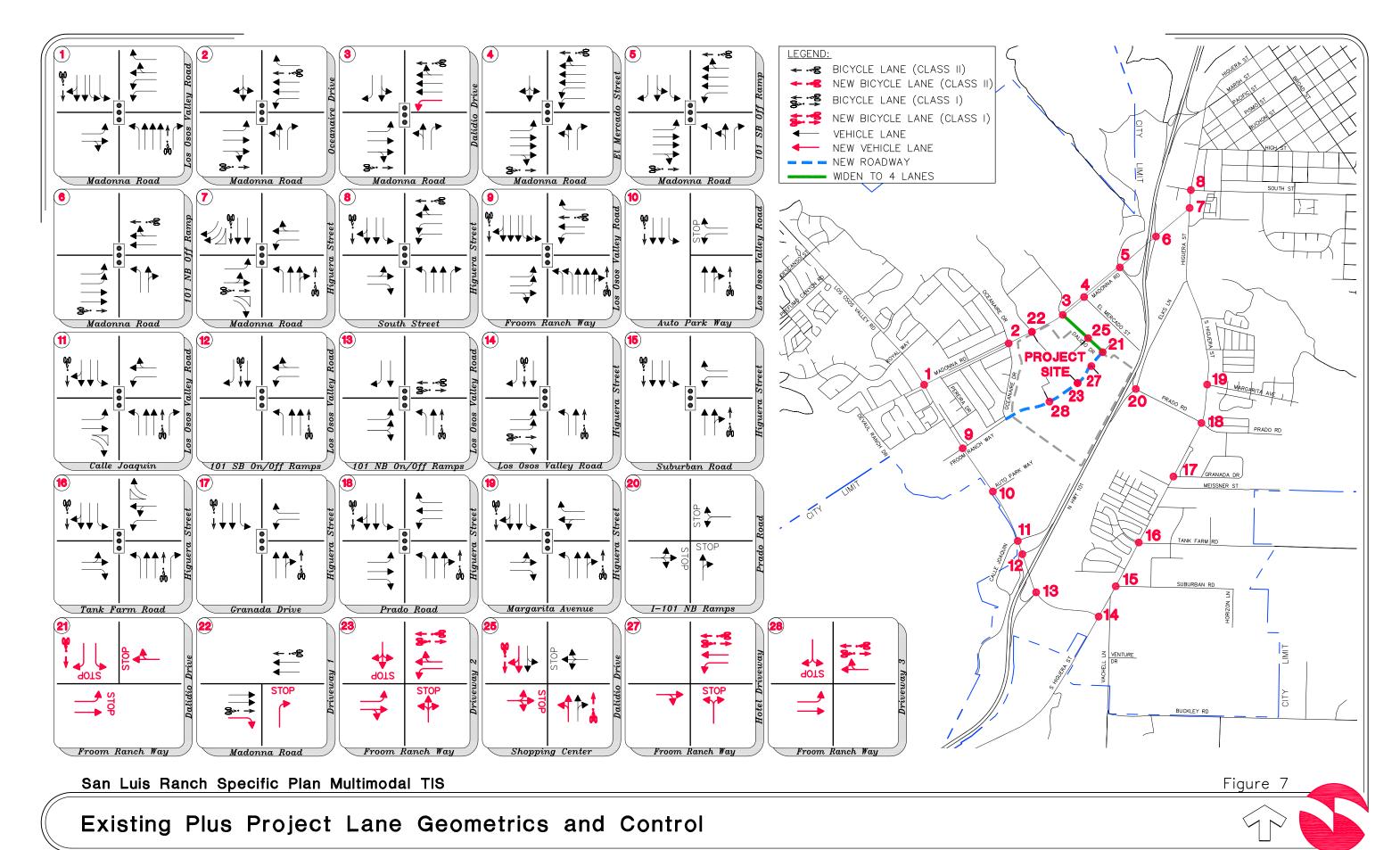


Inset A: Internal Capture Distribution

The inbound and outbound trip distribution for the existing plus project conditions network is shown in Figure 6A and Figure 6B. Figure 7 presents the Existing Plus Project lane geometrics and control. Figure 8 presents the Project Only Peak Hour Traffic Volumes, which is the project trip assignment for the Existing Plus Project and the 2025 Near Term Plus Project conditions.

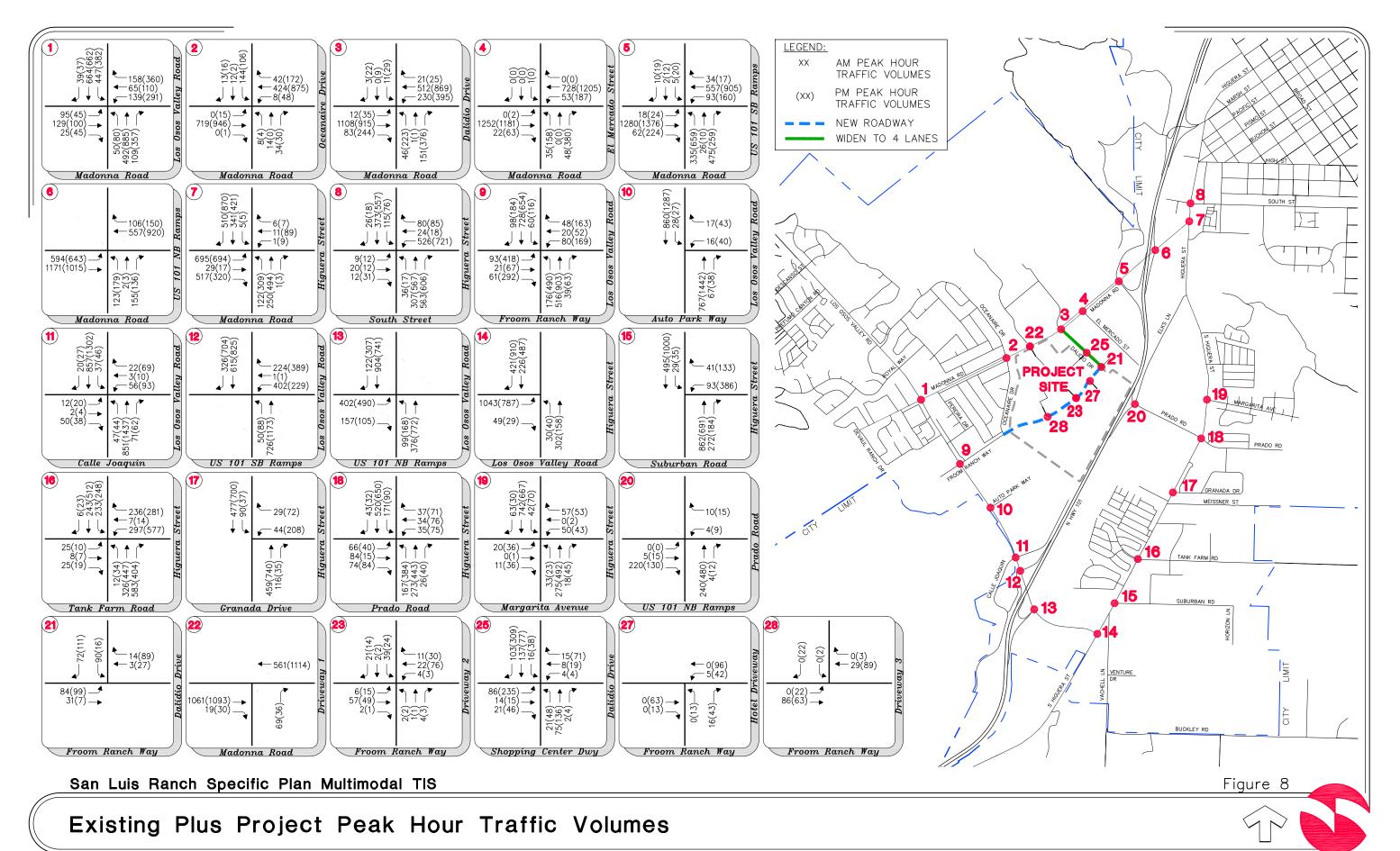






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Existing Plus Project Conditions Analysis

The Existing Plus Project conditions multimodal analysis for the study intersections and segments are presented below.

Existing Plus Project Conditions Intersection Analysis

Table 30 provides a summary of the Existing Plus Project conditions vehicular AM and PM peak hour intersection delay and LOS. Table 31 provides a summary of the Existing Plus Project pedestrian AM and PM peak hour conditions at the study intersections. Table 32 provides a summary of the Existing Plus Project bicycle AM and PM peak hour conditions at the study intersections. Table 33 provides a summary of the Existing conditions queuing analysis.

TABLE 30: EXISTING PLUS PROJECT CONDITIONS INTERSECTION LOS: AUTOMOBILE ANALYSIS

				A	M Peak H	lour	PN	/I Peak H	our
		Control	Target						
#	Intersection	Type 1,2	LOS	v/c³	Delay	LOS	v/c³	Delay	LOS
1	Madonna Road/Los Osos Valley Road	Signal	D		27.3	С		49.4	D
2	Madonna Road/Oceanaire Drive	Signal	D		19.7	В		15.5	В
3	Madonna Road/Dalidio Drive	Signal	D		34.7	С	4.65	172.9	F
4	Madonna Road/El Mercado	Signal	D		7.3	Α		21.8	С
5	Madonna Road/US 101 SB Ramps/Madonna	Signal	С	1.36	48.8	D		23.7	С
6	Madonna Road/US 101 NB Ramps	Signal	С		18.1	В		21.1	С
7	Madonna Road/Higuera Street	Signal	D		19.7	В		24.3	С
8	Higuera Street/South Street	Signal	D		21.8	С	1.47	82.8	F
9	Los Osos Valley Road/Froom Ranch Way	Signal	D		22.3	С		40.7	D
10	Los Osos Valley Road/Auto Park Way	TWSC	D		15.5	С	0.39	36.6	Е
11	Los Osos Valley Road/Calle Joaquin	Signal	D		4.7	Α		5.8	Α
12	Los Osos Valley Road/US 101 SB Ramps	Signal	С		13.2	В		22.7	С
13	Los Osos Valley Road/US 101 NB Ramps	Signal	С		29.1	С		22.5	С
14	S. Higuera Street/Los Osos Valley Road	Signal	D		16.3	В		20.0	В
15	S. Higuera Street/Suburban Drive	Signal	D		6.4	Α		11.2	В
16	S. Higuera Street/Tank Farm Road	Signal	D		36.7	D		21.2	С
17	S. Higuera Street/Granada Drive	Signal	D		8.4	Α		10.5	В
18	S. Higuera Street/Prado Road	Signal	D		16.7	В		21.1	С
19	S. Higuera Street/Margarita Avenue	Signal	D		8.3	Α		12.3	В
20	Prado Road/US 101 NB Ramps	AWSC	С		9.0	Α		13.9	В
21	Froom Ranch Road/Dalidio Drive	AWSC	D		8.5	Α		8.4	Α
22	Madonna Road/Project Driveway	TWSC	D		14.4	В		13.9	В
23	Froom Ranch Road/Project Driveway #2	TWSC	D		9.3	Α		9.7	Α
25	Dalidio Drive/SC Project Driveway	TWSC	D		12.8	В	0.85	49.3	E
27	Froom Ranch Road/Hotel Project Driveway	TWSC	D		9.2	Α		9.3	Α
28	Froom Ranch Road/Project Driveway #3	TWSC	D		8.8	Α		9.0	Α

Notes:

^{1.} AWSC = All Way Stop Control; TWSC = Two Way Stop Control; RNDBT = Roundabout

^{2.} LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC, Signal, RNDBT

^{3.} Volume to Capacity Ratio (v/c) is for worst movement delay, for unacceptable LOS only

TABLE 31: EXISTING PLUS PROJECT CONDITIONS INTERSECTION LOS: PEDESTRIAN ANALYSIS

				AM Peal	Hour	PM Peal	Hour
			Target	Ped. Crosswalk		Ped. Crosswalk	
¥	Intersection	Approach	LOS	Score	LOS	Score	LOS
		EB WB	C C	2.11 2.93	B C	2.11 3.20	B C
1	Madonna Road/Los Osos Valley Road	NB	С	3.06	С	3.39	С
		SB	С	3.19	С	3.20	С
		EB	C	2.70	В	2.90	Ç
	Madonna Road/Oceanaire Drive	WB	С	3.11	С	3.34	С
	Trade Title Control of the Control o	NB	C C	2.03	В	2.08	В
_		SB EB	С	1.83 3.04	A C	1.88 3.41	A C
		WB	Č	3.07	Č	3.17	č
3	Madonna Road/Dalidio Drive	NB	С	2.49	В	2.85	С
		SB	С	1.98	Α	2.03	В
		EB	Ç	n/a	-	n/a	-
1	Madonna Road/El Mercado	WB NB	C	3.14	С	3.24	С
			C C	2.27	В	2.78	C
_		SB EB	C	1.74 3.08	A C	1.74 3.25	A C
	Madonna Road/US 101 SB Ramps/Madonna	WB	C	n/a	-	n/a	-
5	Inn	NB	С	2.75	В	2.67	В
		SB	С	2.18	В	2.19	В
		EB	С	n/a	-	n/a	-
6	Madonna Road/US 101 NB Ramps	WB	C	2.92	C	2.90	С
	The state of the s	NB	С	1.97	Α	2.05	В
		SB	C	n/a	-	n/a	- C
		EB WB	C	3.04 1.99	C A	2.96 2.01	В
7	Madonna Road/Higuera Street	NB	Č	2.71	В	2.80	c
		SB	С	n/a	-	n/a	_
		EB	С	2.01	В	2.01	В
3	Higuera Street/South Street	WB	C	2.75	В	2.81	С
,	niguera Street/South Street	NB	С	n/a	-	n/a	-
		SB	С	2.50	В	2.57	В
		EB	C	2.49	В	2.76	C B
9	Los Osos Valley Road/Froom Ranch Way	WB NB	C C	2.44 n/a	В -	2.65 n/a	- -
		SB	C	3.07	С	3.26	С
		EB	C	0.07	-	0.20	-
_	1 O V-II D I/A (- D - I - W-	WB	С		-		-
0	Los Osos Valley Road/Auto Park Way	NB	С	n/a	-	n/a	-
		SB	С		-		-
		EB	С	2.48	B B	2.28	B B
1	Los Osos Valley Road/Calle Joaquin	WB NB	C C	2.10 2.99	C	2.19 3.21	C
		SB	C	2.91	C	3.21	C
		EB	C	1.91	A	2.34	В
_	1 O V - III - D I II IO 404 0D D	WB	Č	2.22	В	2.11	В
2	Los Osos Valley Road/US 101 SB Ramps	NB	С	n/a	-	n/a	-
		SB	С	n/a	-	n/a	-
		EB	С	2.45	В	2.58	В
3	Los Osos Valley Road/US 101 NB Ramps	NB	С	n/a	-	n/a	-
_		SB	C	n/a	-	n/a	-
	C. Historia Charatill as Const. Valle Box	EB NB	C C	2.66	В	2.74	В
4	S. Higuera Street/Los Osos Valley Road	SB	C	2.25	В	2.30	В
۲		WB WB	C	n/a 2.15	- В	n/a 2.29	- В
5	S. Higuera Street/Suburban Drive	NB NB	c	2.15	C	3.61	D
	<u> </u>	SB	Č	2.73	В	2.85	Ċ
1		EB	С	2.01	В	2.01	В
6	S. Higuera Street/Tank Farm Road	WB	С	2.92	C C	3.04	C C
	·	NB SB	CC	3.34 2.65	В	3.28 2.78	C
		WB	C	2.05	В	2.13	В
7	S. Higuera Street/Granada Drive	NB	C	n/a	-	n/a	-
		SB	С	2.60	В	2.76	С
1		EB	С	2.35	В	2.41	В
8	S. Higuera Street/Prado Road	WB	С	2.28	В	2.32	В
	- 0.2.2 2222 .220 .000	NB	С	2.71	В	2.95	С
_		SB	C	2.76	<u>C</u>	2.78	C
		EB WB	C C	2.23 2.13	B B	2.07 2.19	B B
9	S. Higuera Street/Margarita Avenue	NB	C	2.75	В	2.19	С
		SB	C	2.73	В	2.80	C
٦		EB	C		-		-
_	Prodo Pondil IS 101 NP Pamas	WB	С	n/a	-	n/a	-
U	Prado Road/US 101 NB Ramps	NB	С	n/a	-	n/a	-
		SB	С	1	_	I	_

Notes.

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective.

HCM 2010 Methodologies for the pedestrian mode at two-way stop-controlled intersections is limited to the uncontrolled crossing. No methodology exists for evaluating pedestrian performance for the stop controlled approach (cross-street). However, it is reasoned that this type of control has negligible influence on pedestrian service along the segment.

TABLE 32: EXISTING PLUS PROJECT CONDITIONS INTERSECTION LOS: BICYCLE ANALYSIS

			_	AM Pea	k Hour	PM Pea	k Hour
#	Intersection	Approach	Target LOS	Bicycle LOS Score	LOS	Bicycle LOS Score	LOS
	mersection	EB	D	3.27	C	3.15	C
		WB	D	3.39	C	4.05	D
	Madonna Road/Los Osos Valley Road	NB	D	1.65	A	2.02	В
						l .	
		SB	D	2.63	B	2.52	B
		EB	D	2.75	В	2.96	C
2	Madonna Road/Oceanaire Drive	WB	D	1.08	Α	1.64	Α
		NB	D	2.76	С	2.73	В
		SB	D	2.23	В	2.15	В
		EB	D	2.26	В	2.15	В
3	Madaga Baad (Dalidia Dai)	WB	D	1.59	Α	1.85	Α
5	Madonna Road/Dalidio Drive	NB	D	1.76	Α	2.41	В
		SB	D	1.34	Α	1.42	Α
		EB	D	1.91	Α	1.81	Α
		WB	D	1.79	Α	2.08	В
ļ	Madonna Road/El Mercado	NB	D	3.19	С	3.93	D
		SB	D	3.03	C	3.03	C
		EB	D	2.11	В	2.20	В
						l .	
,	Madonna Road/US 101 SB Ramps/Madonna	WB	D	1.69	Α	1.89	Α
	Inn	NB	D	n/a	-	n/a	-
		SB	D	2.91	С	2.96	С
		EB	D	2.85	С	2.52	В
3	Madonna Road/US 101 NB Ramps	WB	D	1.69	Α	1.95	Α
		NB	D	n/a	-	n/a	
		EB	D	3.46	С	3.02	С
	Madagaa Baadii Farraa Ci	WB	D	2.43	В	2.59	В
7	Madonna Road/Higuera Street	NB	D	1.70	A	2.07	В
		SB	D	2.22	В	2.59	В
		EB	D	2.70	В	2.73	В
		WB	D	2.70	В	3.05	C
3	Higuera Street/South Street	NB	D			3.29	
				3.01	С	l .	С
		SB	D	1.57	A	1.71	A
		EB	D	3.40	С	4.42	E
9	Los Osos Valley Road/Froom Ranch Way	WB	D	1.95	Α	2.66	В
	,	NB	D	1.75	Α	2.15	В
		SB	D	1.72	Α	1.74	Α
		EB	D		-		-
_		WB	D		-	l ,	-
0	Los Osos Valley Road/Auto Park Way	NB	D	n/a	-	n/a	-
		SB	D		-		-
		EB	D	2.99	С	2.98	С
		WB	D	3.08	C	3.23	C
1	Los Osos Valley Road/Calle Joaquin	NB	D	1.48	A	1.91	A
		SB	D			l .	
		WB	D	0.41	A	0.74	A
_	0 1/11 5 1/110 101 05 5			n/a		n/a	
2	Los Osos Valley Road/US 101 SB Ramps	NB	D	2.71	В	3.07	С
		SB	D	2.39	В	3.25	С
		EB	D	n/a	-	n/a	-
3	Los Osos Valley Road/US 101 NB Ramps	NB	D	1.84	Α	2.61	В
		SB	D	3.49	С	3.47	С
		EB	D	2.00	Α	1.75	Α
4	S. Higuera Street/Los Osos Valley Road	NB	D	1.87	Α	1.63	Α
		SB	D	2.40	В	3.69	D
		WB	D	0.89	А	1.56	Α
5	S. Higuera Street/Suburban Drive	NB	D	2.21	В	1.95	Α
	3	SB	D	1.71	A	2.14	В
		EB	D	2.70	В	2.66	В
		WB	D	2.70	В		C
6	S. Higuera Street/Tank Farm Road					3.00	
		NB	D	2.07	В	2.01	В
		SB	D	1.66	A	1.91	A
	<u> </u>	WB	D	2.63	В	2.99	С
7	S. Higuera Street/Granada Drive	NB	D	1.70	Α	1.88	Α
		SB	D	1.83	Α	1.98	Α
		EB	D	2.37	В	2.22	В
0	C. Llieuwere Chroat/Dreed - D	WB	D	2.70	В	2.90	С
ď	S. Higuera Street/Prado Road	NB	D	1.64	Α	1.99	A
		SB	D	1.88	A	1.91	A
		EB	D	2.47	В	2.55	B
		WB	D	2.47	В	2.68	В
9	S. Higuera Street/Margarita Avenue					l .	
		NB	D	1.58	A	1.79	A
		SB	D	2.13	В	2.06	В
		EB	D		-		-
0	Prado Road/US 101 NB Ramps	WB	D	n/a	-	n/a	-
J	Tado Noduros To FND Nattips	NB	D	11/d	-	IIId	-
			D				

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective.

No methodology exists for evaluating bicycle performance at two-way stop-controlled intersections. However, it is reasoned that this type of control has negligible influence on bicycle service along the segment for stop controll on the cross-street.

TABLE 33: EXISTING PLUS PROJECT CONDITIONS 95TH PERCENTILE QUEUING ANALYSIS

Inter	section			Total	95 th Pe Queue/I	rcentile Lane (ft)
ID	Location	Movement	No. Lanes	Storage (ft) ¹	AM Peak Hour	PM Peak Hour
1	Madonna Road/Los	Northbound Right	1	175	92	250
2	Madonna Madonna	Westbound Right	1	100	46	165
	Madonna Road/Dalidio	Eastbound Left	1	115	65	125
3	Drive	Westbound Left	1	275	224	376
5	Madonna Road/US 101	Eastbound Left	1	100	72	117
6	Madonna Road/US 101	Northbound Left	1	185	141	192
	Madonna Road/Higuera	Eastbound Right	1	150	325	265
7	Street	Northbound Left	1	160	121	252
		Westbound Left	2	240	214	389
	Higuera Street/South	Northbound Left	1	60	88	72
8	Street	Northbound Right	1	60	149	150
	0.1001	Southbound Left	1	70	130	107
9	Los Osos Valley	Westbound Right	1	50	56	87
U	,	Northbound Through	2	235	139	135
11	Los Osos Valley	Southbound Left	1	180	217	209
• • •	Road/Calle Joaquin	Southbound Through/Right	2	-	945	722
		Westbound Left	1	150	242	227
	Los Osos Valley	Northbound Left	1	80	101	117
12	Road/US 101 SB	Southbound Through	1	240	308	279
	Ramps	Southbound Right	1	240	220	262
		Eastbound Left/Right	1	200	228	246
	Los Osos Valley	Northbound Left	1	140	109	200
13	Road/US 101 NB	Southbound Through	1	865	1119	811
	Ramps	Southbound Right	1	60	204	221
14	S. Higuera Street/Los	Eastbound Right	1	90	165	136
40	S. Higuera Street/Tank	Northbound Right	1	100	194	123
16	Farm Road	Southbound Left	1	165	89	215
		Northbound Left	1	100	132	170
40	S. Higuera Street/Prado	Northbound Through/Right	2	-	134	379
18	Road	Southbound Left	1	60	117	108
		Southbound Through/Right	2	-	230	237
19	S. Higuera	Southbound Left	1	60	51	75
	Street/Margarita Avenue					L

Not 1. Bolded entries indicate queues exceed available storage

- 2. Storage Length of " " represents a lane which exceeds 900 feet, usually a through lane.
- 3. For Movements with more than one lane, the maximum of the 95th percentile queue is reported.
- 4. * Represents storage lengths for one lane; second lane is a left or right trap lane.

As shown in Table 30, the intersection of Madonna Road/Dalidio Drive, Madonna Road/US 101 SB Ramps/Madonna Inn, Higuera Street/South Street, and Los Osos Valley Road/Auto Park Way continue to operate at unacceptable conditions during AM and PM peak hours under Existing Plus Project Conditions. The northbound and southbound approaches experience long delays. Queuing analysis results show numerous locations in which the 95th percentile queue exceeds the available storage capacity during existing conditions.

Pedestrian and bicycle analysis shows acceptable conditions at the study intersections, except for the intersections of Los Osos Valley Road/US 101 SB Ramps and Los Osos Valley

Road/Froom Ranch Way, respectively, which continue to operate at unacceptable conditions under Existing Plus Project Conditions. Transportation improvements required to maintain acceptable multi-modal conditions are detailed in a subsequent section of this report.

Existing Plus Project Conditions Segment Analysis

Table 34 provides a summary of the Existing Plus Project vehicular AM and PM peak hour conditions for the study segments. Table 35 provides a summary of the Existing Plus Project pedestrian AM and PM peak hour conditions for the study segments. Table 36 provides a summary of the Existing Plus Project bicycle AM and PM peak hour conditions for the study segments. Table 37 provides a summary of the Existing Plus Project transit AM and PM peak hour conditions for the study segments. Table 38 provides a summary of the Existing Plus Project freeway segments analysis for AM and PM peak hour conditions for the study segments along US 101. Transportation improvements required to mitigate project-related impacts are detailed in a subsequent section of this report.

TABLE 34: EXISTING PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: AUTOMOBILE ANALYSIS

UTO SEGMENT LOS						AM P	EAK			PM P		
				LOS					Travel	Base Free-	Travel	
				Threshold	Travel Speed		Travel Speed/		Speed	Flow Speed	Speed/	
D Roadway	From	То	Direction		(mph)	Speed BFFS (mph)	BFFS (%)	LOS	(mph)	BFFS (mph)	BFFS (%)	LC
Madonna Rd	Oceanaire Dr	LOVR	WB	D	21.0	42.1	50%	D	10.8	42.1	26%	I
Madonna Rd	LOVR	Oceanaire Dr	EB	D	29.1	42.1	69%	В	31.0	42.1	74%	
2 Madonna Rd	Dalidio	Oceanaire Dr	WB	D	22.7	40.8	56%	С	24.6	40.7	60%	
Madonna Rd	Oceanaire Dr	Dalidio	EB	D	15.5	40.7	38%	E	13.4	40.8	33%	
B Madonna Rd	El Mercado	Dalidio Dr	WB	D	17.1	34.8	49%	D	16.0	34.8	46%	
Madonna Rd	Dalidio Dr	El Mercado	EB	D	20.9	34.7	60%	С	12.7	34.6	37%	
Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	31.9	37.9	84%	В	20.7	37.3	55%	
Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	22.3	37.8	59%	С	17.5	37.7	46%	
Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	34.2	37.8	90%	Α	33.8	37.8	89%	
Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	32.5	37.8	86%	Α	33.3	37.8	88%	
Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	9.4	37.2	25%	F	11.0	37.2	30%	
Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	16.5	37.2	44%	D	12.9	37.2	35%	
S. Higuera St	Madonna Rd	Margarita Ave	SB	D	37.1	44.5	83%	В	35.3	44.5	79%	
S. Higuera St	Margarita Ave	Madonna Rd	NB	D	35.6	44.8	79%	В	35.7	44.8	80%	
S. Higuera St	Margarita Ave	Prado Rd	SB	D	18.6	38.9	48%	D	16.5	38.9	42%	
S. Higuera St	Prado Rd	Margarita Ave	NB	D	25.4	38.9	65%	С	21.0	38.9	54%	
S. Higuera St	Prado Rd	Granada Dr	SB	D	33.8	41.8	81%	В	30.6	41.8	73%	
S. Higuera St	Granada Dr	Prado Rd	NB	D	25.5	41.9	61%	С	28.1	41.9	67%	
S. Higuera St	Granada Dr	Tank Farm Road	SB	D	42.6	41.6	102%	A	27.3	42.6	64%	
S. Higuera St	Tank Farm Road	Granada Dr	NB	D	30.5	41.6	73%	В	28.7	42.6	67%	
S. Higuera St	Tank Farm Road	Suburban Drive	SB	D	27.3	42.4	65%	C	24.8	41.2	60%	
S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	20.2	42.5	47%	D	18.9	41.3	46%	
S. Higuera St	Suburban Drive	Los Osos Vallev Road	SB	D	20.6	42.1	49%	D	15.8	39.1	40%	
S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	D	24.7	42.0	59%	C	21.5	39.0	55%	
Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	23.3	41.9	56%	C	17.2	41.8	41%	
Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	20.3	41.8	49%	D	16.7	41.8	40%	
Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	34.6	43.0	81%	B	30.9	43.0	72%	
Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	29.2	43.2	68%	В	22.4	43.0	52%	
Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	D	13.2	32.1	41%	D	8.3	32.1	26%	
Los Osos Valley	•	•	NB	D	16.9	31.1	54%	C	15.2	31.1	49%	
	US 101 SB Ramps	Calle Joaquin		D		37.7		F				
Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB		11.0		29%		12.9	37.7	34%	
Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	D	32.6	37.4	87%	Α	31.8	37.4	85%	
7 Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	D	20.6	39.2	53%	С	17.7	39.2	45%	
Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	D	29.1	39.2	74%	В	25.5	39.2	65%	
Prado Rd	S. Higuera St	US 101 NB Ramps	WB	D	28.0	38.3	73%	В	24.7	38.3	64%	
Prado Rd	US 101 NB Ramps	S. Higuera St	EB	D	23.7	38.3	62%	С	21.2	38.3	55%	
Froom Ranch Way	Dick's Sporting Goods Dwy	Los Osos Valley	WB	D	17.7	38.0	46%	D	12.8	38.0	34%	
Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods Dwy	EB	D	34.7	37.4	93%	Α	34.5	37.4	92%	
Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	D	29.3	39.3	74%	В	29.5	39.3	75%	
Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	D	6.3	39.4	16%	F	1.6	39.4	4%	
1 Froom Ranch Way	Dalidio	Dick's Sporting Goods Dwy	WB	D	40.0	40.6	98%	Α	39.9	40.6	98%	
Froom Ranch Way	Dick's Sporting Goods Dwy	Dalidio	EB	D	34.1	40.8	84%	В	33.9	40.8	83%	

TABLE 35: EXISTING PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: PEDESTRIAN ANALYSIS

PEDESTRIAN SEGMENT LOS						AM PEAK		PM PEAK	
				LOS	Average Ped.	SEGMENT		SEGMENT	
ID Roadway	From	То	Direction	Threshold	Space (ft²/p)	SCORE	LOS	SCORE	LOS
1 Madonna Rd	Oceanaire Dr	LOVR	WB	С	2648	3.56	D	3.84	D
Madonna Rd	LOVR	Oceanaire Dr	EB	С	17482	3.76	D	3.90	D
2 Madonna Rd	Dalidio	Oceanaire Dr	WB	С	12000	3.60	D	3.92	D
Madonna Rd	Oceanaire Dr	Dalidio	EB	С	5833	3.87	D	3.97	D
3 Madonna Rd	El Mercado	Dalidio Dr	WB	С	37450	3.59	D	3.92	D
Madonna Rd	Dalidio Dr	El Mercado	EB	С	52920	3.70	D	3.86	D
4 Madonna Rd	US 101 SB Ramps	El Mercado	WB	С	26250	3.66	D	3.84	D
Madonna Rd	El Mercado	US 101 SB Ramps	EB	С	27915	3.93	D	4.17	D
5 Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	С	No Peds	3.73	D	3.90	F
Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	С	No Peds	4.15	D	4.10	D
6 Madonna Rd	Higuera St	US 101 NB Ramps	WB	С	25200	3.65	D	3.81	D
Madonna Rd	US 101 NB Ramps	Higuera St	EB	С	19838	3.90	D	3.79	D
7 S. Higuera St	Madonna Rd	Margarita Ave	SB	c	23247	3.82	D	3.80	D
S. Higuera St	Margarita Ave	Madonna Rd	NB	С	5398	3.61	D	3.80	D
8 S. Higuera St	Margarita Ave	Prado Rd	SB	С	40979	3.61	D	3.63	D
S. Higuera St	Prado Rd	Margarita Ave	NB	С	21700	3.45	С	3.58	D
9 S. Higuera St	Prado Rd	Granada Dr	SB	С	9292	3.55	D	3.66	D
S. Higuera St	Granada Dr	Prado Rd	NB	С	8400	3.16	С	3.38	С
10 S. Higuera St	Granada Dr	Tank Farm Road	SB	C	46305	3.54	D	3.69	D
S. Higuera St	Tank Farm Road	Granada Dr	NB	C	49140	3.11	С	3.26	С
11 S. Higuera St	Tank Farm Road	Suburban Drive	SB	C	12600	3.57	D	3.80	D
S. Higuera St	Suburban Drive	Tank Farm Road	NB	C	31500	3.48	c	3.44	C
12 S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	Č	39312	3.57	D	3.85	D
S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	c	43533	3.85	D	3.89	D
13 Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	С	3853	3.81	D	3.88	D
Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	С	l o	3.70	F	4.05	F
14 Los Osos Vallev	Froom Ranch Way	Calle Joaquin	SB	С	27300	3.77	D	4.01	D
Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	c	22050	3.68	D	4.00	D
15 Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	С	No Peds	3.61	D	3.70	D
Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	c	63000	3.63	D	3.93	D
16 Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	c	No Peds	4.17		4.22	D
Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	c	53928	3.63	D	3.13	c
17 Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	c	46575	4.23	D	4.02	D
Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	c	1680	3.68	D	4.09	D
18 Prado Rd	S. Higuera St	US 101 NB Ramps	WB	С	25200	2.90	С	2.90	С
Prado Rd	US 101 NB Ramps	S. Higuera St	EB	С	3019	3.42	С	3.32	С
19 Froom Ranch Way	Dick's Sporting Goods Dwy	Los Osos Valley	WB	С	4500	3.03	С	3.28	С
Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods Dwy	EB	С	7350	1.76	Α	1.79	A
20 Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	C	3500	1.64	A	1.61	A
Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	c	21000	3.14	c	3.40	c
21 Froom Ranch Way	Dalidio	Dick's Sporting Goods Dwy	WB	C	2520	1.55	A	1.57	A
Froom Ranch Way	Dick's Sporting Goods Dwy	Dalidio Dalidio	EB	C	No Peds	1.64	A	1.64	A
Notes:	_ consuperting codes bwy					1.0.		1.0.	

Notes:

Sidewalk is present along frontage roads for segments #1 - Madonna Road and #13 - Los Osos Valley Road, and is not accounted for in this analysis.

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. No methodology exists for evaluating two-way stop-controlled interesction performance (with the cross-street stop controlled) for pedestrians and bicycles. However, it is reasoned that it has negligible influence on pedestrian sevice along the segment.

TABLE 36: EXISTING PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: BICYCLE ANALYSIS

BIC	YCLE SEGMENT LOS					AM PE	AK	PM PEA	AK
ID	Roadway	From	То	Direction	LOS Threshold	SEGMENT SCORE	LOS	SEGMENT SCORE	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	D	3.61	D	3.98	D
	Madonna Rd	LOVR	Oceanaire Dr	EB	D	3.74	D	3.80	D
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	D	3.13	С	3.22	С
	Madonna Rd	Oceanaire Dr	Dalidio	EB	D	3.58	D	3.44	С
3	Madonna Rd	El Mercado	Dalidio Dr	WB	D	3.30	С	3.22	С
	Madonna Rd	Dalidio Dr	El Mercado	EB	D	3.51	D	3.41	С
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	3.97	D	4.36	
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	3.64	D	3.66	D
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	3.32	С	3.37	С
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	3.42	С	3.36	С
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	3.50	D	3.56	D
	Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	3.73	D	3.59	D
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	D	3.89	D	3.80	D
	S. Higuera St	Margarita Ave	Madonna Rd	NB	D	4.06	D	4.15	D
8	S. Higuera St	Margarita Ave	Prado Rd	SB	D	3.64	D	3.64	D
	S. Higuera St	Prado Rd	Margarita Ave	NB	D	3.87	D	3.87	D
9	S. Higuera St	Prado Rd	Granada Dr	SB	D	3.84	D	3.87	D
	S. Higuera St	Granada Dr	Prado Rd	NB	D	3.42	С	3.50	С
10	S. Higuera St	Granada Dr	Tank Farm Road	SB	D	4.10	D	4.16	D
	S. Higuera St	Tank Farm Road	Granada Dr	NB	D	3.48	С	3.52	D
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	D	3.33	С	3.41	С
	S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	3.40	С	3.39	С
12	S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	D	3.24	С	3.60	D
	S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	D	3.90	D	3.86	D
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	3.71	D	3.71	D
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	3.38	С	3.46	С
14	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	3.56	D	3.59	D
	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	3.81	D	3.89	D
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	D	3.34	С	3.54	D
	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	D	3.54	D	3.60	D
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	D	3.73	D	3.73	D
	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	D	3.40	С	3.51	D
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	D	3.45	С	3.41	С
	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	D	3.29	С	3.43	С
18	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	D	3.44	С	3.44	С
	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	D	3.94	D	3.68	D
19	Froom Ranch Way	Dick's Sporting Goods Dwy	Los Osos Valley	WB	D	3.08	С	3.55	D
	Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods Dw	EB	D	3.24	С	2.49	В
20	Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	D	3.35	С	3.05	С
	Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	D	3.32	С	3.43	С
21	Froom Ranch Way	Dalidio	Dick's Sporting Goods Dw	WB	D	2.50	В	2.92	С
	Froom Ranch Way	Dick's Sporting Goods Dwy	Dalidio	EB	D	3.42	С	3.41	С

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. No methodology exists for evaluating two-way stop-controlled intersection performance (with the cross-street stop controlled) for pedestrians and bicycles. However, it is incorporated into the methodology for evaluating bicycle segment performance.

TABLE 37: EXISTING PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: TRANSIT ANALYSIS

TRANSIT LOS						AM PEA	ıK	PM PEAK	
				LOS	Route Name	SEGMENT		SEGMENT	
ID Roadway	From	То	Direction	Threshold		SCORE	LOS	SCORE	LOS
1 Madonna Rd	Oceanaire Dr	LOVR	WB	D	Ruote 4	4.17	D	4.30	E
Madonna Rd	LOVR	Oceanaire Dr	EB	D	Route 5	4.30	E	3.98	D E
2 Madonna Rd Madonna Rd	Dalidio Oceanaire Dr	Oceanaire Dr Dalidio	WB EB	D	Route 4 Route 5	4.46 4.74	E E	4.57 4.54	E
3 Madonna Rd	El Mercado	Dalidio Dr	WB	D D	Route 5	4.74	E	4.54	E
Madonna Rd	Dalidio Dr	El Mercado	EB	D D	Route 5	Not Analyzed	N/A	Not Analyzed	N/A
4 Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	Route 4	4.36	E	4.59	E
Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	Route 5	4.67	E	4.48	E
5 Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	Route 4	3.97	D	4.09	D
Madonna Rd	US 101 NB Ramps	US 101 NB Ramps	EB	D	Route 5	4.35	E	4.04	D
6 Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	Route 4	4.33	E	4.44	E
	•		EB	D		4.55	E		E
Madonna Rd	US 101 NB Ramps	Higuera St		D	Route 5			4.29	E
7 S. Higuera St	Madonna Rd	Margarita Ave	SB	D	Route 2	Not Analyzed	N/A	3.51	
S. Higuera St	Margarita Ave	Madonna Rd	NB		Route 2	3.65	D	3.69	D
8 S. Higuera St	Margarita Ave	Prado Rd	SB	D	Route 2	Not Analyzed	N/A	4.19	D
S. Higuera St	Prado Rd	Margarita Ave	NB	D	Route 2	4.15	D	4.23	D
9 S. Higuera St	Prado Rd	Granada Dr	SB	D	Route 2	4.35	E	4.28	E
S. Higuera St	Granada Dr	Prado Rd	NB	D	Route 2	3.76	D	3.83	D
10 S. Higuera St	Granada Dr	Tank Farm Road	SB	D	Route 2	3.75	D	3.83	D
S. Higuera St	Tank Farm Road	Granada Dr	NB	D	Route 2	3.51	D	3.58	D
11 S. Higuera St	Tank Farm Road	Suburban Drive	SB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	Route 2	3.97	D	3.96	D
12 S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
13 Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	Route 4	4.50	E	4.53	E
Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 4	4.22	D	4.38	E
13 Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	Route 5	4.61	E	4.38	E
Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 5	4.36	E	4.21	D
14 Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	Route 4	4.22	D	4.35	E
Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	Route 4	4.24	D	4.49	E
14 Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	Route 5	4.37	E	4.16	D
Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	Route 5	4.37	E	4.33	E
15 Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	D		Not Analyzed	N/A	Not Analyzed	N/A
16 Los Osos Valley		US 101 NB Ramps	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
Los Osos Valley	•	US 101 SB Ramps	NB	D		Not Analyzed	N/A	Not Analyzed	N/A
17 Los Osos Valley		S. Higuera St	EB	D		Not Analyzed	N/A	Not Analyzed	N/A
Los Osos Valley	•	US 101 NB Ramps	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
18 Prado Rd	S. Higuera St	US 101 NB Ramps	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
Prado Rd	US 101 NB Ramps	S. Higuera St	EB	D	Route 2	3.83	, D	Not Analyzed	N/A
19 Froom Ranch W		Los Osos Valley	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
Froom Ranch W		Dick's Sporting Goods Dwy	EB	D		Not Analyzed	N/A	Not Analyzed	N/A
20 Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	D	Route 4	4.05	D	4.08	D
Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	D	Route 5	4.20	D	4.03	D
21 Froom Ranch W		Dick's Sporting Goods Dwy	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
Froom Ranch W	•	Dalidio	EB	D		Not Analyzed	N/A	Not Analyzed	N/A

Segment 20 transit is southbound for routes 4 and 5 $\,$

Route 2 Serves the Prado Day Center stop during the AM peak hour, and the DMV/Margarita stop during the PM Peak Hour

TABLE 38:
EXISTING PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: FREEWAY ANALYSIS

				AM	Peak Hour		PM	Peak Hour	
	Target	Segment	No. of		Density			Density	
Interchange Location	LOS	Type	Lanes	Volume	(pc/mi/ln)	LOS	Volume	(pc/mi/ln)	LOS
US 101 at Los Osos Valley Road									
US 101 NB South of Los Osos Valley Road	С	Freeway	2	2,801	24.8	С	2,284	20.0	С
US 101 SB South of Los Osos Valley Road	С	Freeway	2	1,432	12.6	В	3,082	27.8	D
US 101 at Prado Road				•					
US 101 NB South of Prado Road	С	Freeway	2	2,463	21.6	С	2,164	19.0	С
US 101 at Madonna Road				•		-			
US 101 NB South of Madonna Road	С	Freeway	2	2,488	21.8	С	2,524	22.2	С
US 101 SB South of Madonna Road	С	Freeway	2	1,682	14.8	В	2,908	25.9	С

As shown in the Existing Plus Project Conditions segment analysis tables, most of the pedestrian and transit facilities are projected to operate at unacceptable levels of service as well as several segments for automobile mode and only one segment for the bicycle mode.

Existing Plus Project Impacts & Mitigation Measures

This section presents the project-related impacts and mitigation measures at the study intersections and segments, developed based on the findings from the analyses presented in the prior sections of this report. Improvements are identified only where this is a significant project impact, based on the significance thresholds identified previously.

Table 39 presents the intersections projected to operate at unacceptable levels of service for vehicle, pedestrian, bicycle, and transit travel modes under *Existing Plus Project* conditions, and whether the project has a significant impact based on the City's thresholds. Tables 40A and 40B present the roadway segments projected to operate at unacceptable levels of service for vehicle, pedestrian, bicycle, and transit travel modes under Existing Plus Project AM and PM peak hour conditions, respectively, and whether the project has a significant impact based on the City's thresholds. Table 40C presents the project impacts for the queuing analysis at the study intersections.

TABLE 39: EXISTING PLUS PROJECT SIGNIFICANT INTERSECTION IMPACTS

Intersection #	3	}		5		8	9		1	0	1	5	25	
Name	Madonn Dalidic		Madonn US 10 Rar		_	a Street/ Street	Los Oso Road/ I Ranch	Froom	Los Oso Road/ Ai	uto Park		et/	Dalidio Drive/ Drivew	-
Mode	AU	ТО	AU	ТО	AU	ITO	BIŁ	Œ	AU	ТО	PE	D	AUT	0
SCENARIO	v/c	LOS	v/c	LOS	v/c	LOS	Score	LOS	v/c	LOS	v/c	LOS	v/c	LOS
AM PEAK HOUR							EB				NB			
Existing	-	Α	1.31	D	-	С	3.38	С	-	В	2.98	С	-	Α
Existing Plus Project	-	С	1.36	D	-	С	3.40	С	-	С	2.99	С	-	В
Significant Impact?	N	0	Ye	es	١	10	N	0	N	0	N	0	No	
PM PEAK HOUR							EB				NB			
Existing	2.11	E	_	С	1.31	E	4.39	Ε	-	D	3.60	D	-	Α
Existing Plus Project	4.65	F	_	С	1.47	F	4.42	Ε	0.39	Ε	3.61	D	0.85	E
Significant Impact?	Ye	es	N	lo	Y	es	N	0	Ye	es	N	0	Yes	

Note: V/C Ratio is based on worst movement

TABLE 40A: EXISTING PLUS PROJECT SIGNIFICANT ROADWAY IMPACTS

Existing Plus P	roject Multimodal S	Segment LOS				Aut	o Mode					Pedestr	ian Mode					Bicyc	le Mode					Trans	sit Mode		
AM PEAK HOU	R				Existi	ng	Existing Plu	ıs Project			Existir	ng	Existing Plus	s Project			Existi	ng	Existing Plus	s Project			Existing		Existing Plus P	roject	
				Auto LOS Threshold			Travel Speed		nt	Ped LOS Threshold	Segment		Segment		Significa nt		Segment		Segment		Significa nt	Transit LOS Threshold					Significant
ID Roadway	From	То	Direction		(mph)	LOS	(mph)	LOS	Impact?		Score	LOS	Score	LOS	Impact?		Score	LOS	Score	LOS	Impact?	moonora	Segment Score	LOS	Segment Score	LOS	Impact?
2 Madonna Rd	Oceanaire Dr	Dalidio	EB	D	27.1	С	15.5	E	Yes	С	3.80	D	3.87	D	No	D	3.57	D	3.58	D	No	D	4.70	Е	4.74	E	No
3 Madonna Rd	El Mercado	Dalidio Dr	WB	D	21.2	С	17.1	D	No	С	3.52	D	3.59	D	No	D	3.27	С	3.30	С	No	D	4.24	D	4.35	E	Yes

Note: Segment 20 transit is southbound for routes 4 and 5; Segment 21 pedestrian and bicycle service will be further evaluated using off-street facilities methodologies.

Existing Plus Pro	oject Multimodal Seg	gment LOS				Au	to Mode					Pedest	rian Mode					Bicyc	le Mode					Trans	sit Mode		
PM PEAK HOUR					Existi	ng	Existing Plu	ıs Project			Exist	ing	Existing Plu	ıs Project			Existi	ng	Existing Plus	Project			Existing		Existing Plus P	roject	
				Auto LOS	_		Travel		Significa				0		Significa				0		Significa	Transit LOS					G::
ID. Decelor	F	т.	Di	Threshold		1.00	Speed	1.00		Inresnoid	Segment		Segment	1.00	nt	Inresnoid	Segment	1.00	Segment	1.00	nt	Threshold	0	1.00	0		Significant
ID Roadway	From	10	Direction		(mph)	LOS	(mph)	LOS	Impact?		Score	LOS	Score	LOS	Impact?		Score	LOS	Score	LOS	Impact?		Segment Score	LOS	Segment Score	LOS	Impact?
1 Madonna Rd	Oceanaire Dr	LOVR	WB	D	12.7	E	10.7	F	Yes	С	3.79	D	3.82	D	No	D	3.93	D	3.97	D	No	D	4.28	Е	4.32	E	No
2 Madonna Rd	Oceanaire Dr	Dalidio	EB	D	18.9	D	13.4	Е	Yes	С	3.89	D	3.97	D	No	D	3.43	С	3.44	С	No	D	4.51	Е	4.54	E	No
6 Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	13.3	Е	12.9	Е	No	С	3.72	D	3.79	D	No	D	3.53	D	3.59	D	No	D	4.18	D	4.29	E	Yes
13 Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	16.9	D	16.7	E	Yes	С	4.04	F	4.05	F	No	D	3.46	С	3.46	С	No	D	4.38	E	4.38	E	No

Note: Segment 20 transit is southbound for routes 4 and 5; Segment 21 pedestrian and bicycle service will be further evaluated using off-street facilities methodologies.

TABLE 40C: EXISTING PLUS PROJECT SIGNIFICANT QUEUING IMPACTS

					Exis	sting	EXPP Co	onditions
	Intersection		No.	Total		rcentile Lane (ft)		rcentile _ane (ft)
ID	Location	Movement	Lanes	Storage (ft) ¹	AM Peak Hour	PM Peak Hour	AM Peak Hour	PM Peak Hour
1	Madonna Road/Los Osos Valley Road	Northbound Right	1	175	96	240	92	250
2	Madonna Road/Oceanaire Drive	Westbound Right	1	100	39	95	46	165
3	Madonna Road/Dalidio Drive	Westbound Left	1	275	42	134	224	376
5	Madonna Road/US 101 SB Ramps/Madonna Inn	Eastbound Left	1	100	92	94	72	117
6	Madonna Road/US 101 NB Ramps	Northbound Left	1	185	144	178	141	192
7	Madama Dood/I limitara Chroch	Eastbound Right	1	150	232	146	325	265
,	Madonna Road/Higuera Street	Northbound Left	1	160	107	251	121	252
		Westbound Left	2	240	219	310	214	389
0	Lieuwaya Chrach/Cautha Chrach	Northbound Left	1	60	87	74	88	72
8	Higuera Street/South Street	Northbound Right	1	60	142	136	149	150
		Southbound Left	1	70	109	97	130	107
0	Los Osos Valley Road/Froom	Eastbound Left	2*	250	108	309	103	336
9	Ranch Way	Westbound Right	1	50	41	82	56	87
11	Los Osos Valley Road/Calle Joaquin	Southbound Left	1	180	108	170	217	209
		Westbound Left	1	150	241	224	242	227
12	Los Osos Valley Road/US 101 SB Ramps	Southbound Through	2	240	297	289	308	279
		Southbound Right	1	240	177	238	220	262
12	Los Osos Valley Road/US 101 NB	Southbound Through	1	940	1042	822	1119	811
13	Ramps	Southbound Right	1	135	186	219	204	221
14	S. Higuera Street/Los Osos Valley Road	Eastbound Right	1	90	164	122	165	136
16	S. Higuera Street/Tank Farm Road	Northbound Right	1	100	137	134	146	123
10	S. Higuera Street/Prado Road	Northbound Left	1	100	131	176	132	170
18	S. Filguera Suleet/Prado Road	Southbound Left	1	60	116	109	117	108
19	S. Higuera Street/Margarita Avenue	Southbound Left	1	60	58	67	51	75

Notes: 1. Bolded entries indicate queues exceed available storage

2. Storage Length of " - " represents a lane which exceeds 900 feet, usually a through lane.

4. * Represents storage lengths for one lane; second lane is a left or right trap lane.

Red Cells represent New Impacts from Project-added traffic

Yellow Cells represent Existing queues exceeding storage that are increased by project-added traffic.

^{3.} For Movements with more than one lane, the maximum of the 95th percentile queue is reported.

Existing Plus Project Mitigation Measures

This section presents project impacts and potential mitigation measures at the study locations. The feasibility of these improvements is not known beyond a preliminary planning level. Where significant impacts to intersections and for roadway segments have been identified, mitigation measures have been proposed to reduce the project impact to less than significant. The mitigation measures have been summarized in Table 41, identifying which mitigation measures are projected to reduce the project's impact to less than significant for the corresponding intersection or segment, for each mode. Additionally, it is important to note that the optimization of throughput to mitigate segment impacts may create queuing impacts for opposing movements.

TABLE 41: EXISTING PLUS PROJECT MITIGATION MEASURES SUMMARY

Existing Plus Project Significant Impact Location	Mode	Impact	Mitigation Measure
Intersection #1 Madonna Rd/Los Osos Valley Rd	Auto (Queue)	Addition of project traffic would exacerbate the existing northbound right queue in the PM peak hour.	Construction of the Prado Overcrossing would improve this condition to an acceptable level.
Intersection #2 Madonna Rd/Oceanaire Dr	Auto (Queue)	Addition of project traffic would result in a new impact to the westbound right queue in the PM peak hour.	Construction of the Prado Overcrossing would improve this condition to an acceptable level.
Intersection #3 Madonna Rd/Dalidio Dr	Auto	This intersection currently operates at unacceptable LOS E in the PM peak hour. The addition of Project traffic exacerbates this unacceptable condition by increasing delay and v/c ratio.	 Install a second westbound left turn pocket with 310' storage on Madonna Road and extend current pocket to 310'; this may require additional right-of-way, potentially impacting private or public land. Install dedicated eastbound right turn pocket on Madonna Road. Provide northbound right turn overlap phase, and prohibit westbound U-turns. Provide a split phase for northbound and southbound. Optimize traffic signal cycle lengths.

TABLE 41: EXISTING PLUS PROJECT MITIGATION MEASURES SUMMARY

Existing Plus Project Significant Impact Location	Mode	Impact	Mitigation Measure
Intersection #5 Madonna Rd/US 101 SB Ramps/Madonna Inn	Auto	This intersection currently operates at unacceptable LOS D in the AM peak hour. The addition of Project traffic exacerbates this unacceptable condition by increasing delay and v/c ratio.	Construction of the Prado Overcrossing would improve this condition to an acceptable level.
Intersection #5 Madonna Rd/US 101 SB Ramps/Madonna Inn	Auto (Queue)	Addition of project traffic would result in a new impact to the eastbound left queue in the PM peak hour.	Extension of the eastbound left turn pocket to 150' would improve this condition to an acceptable level. This pocket is back-to-back with another turn pocket that would be shortened to accommodate the extension with no residual impact.
Intersection #6 Madonna Rd/US 101 NB Ramps	Auto (Queue)	Addition of project traffic would result in a new impact to the northbound left queue in the PM peak hour.	Construction of the Prado Overcrossing would improve this condition to an acceptable level.
Intersection #7 Madonna Rd/S. Higuera St	Auto (Queue)	Addition of project traffic would: •Exacerbate the existing eastbound right queues in the AM and result in a new impact in the PM peak hour. •Exacerbate the northbound left queue in the PM peak hour.	Construction of the Prado Overcrossing with Northbound Ramps would improve this condition to an acceptable level.
Intersection #8 Higuera St/South St	Auto	This intersection currently operates at unacceptable LOS E in the PM peak hour. The addition of Project traffic exacerbates this unacceptable condition by increasing delay and v/c ratio, resulting in LOS F.	Optimize signal timing to accommodate increased volume will improve the operations at this intersection.

TABLE 41: EXISTING PLUS PROJECT MITIGATION MEASURES SUMMARY

Existing Plus Project Significant Impact Location	Mode	Impact	Mitigation Measure
Intersection #9 Los Osos Valley Road/Froom Ranch Way	Auto (Queue)	Addition of project traffic would: •Exacerbate the eastbound left queue in the PM peak hour. •Result in a new impact in the AM peak hour and exacerbate the westbound right queue in the PM peak hour.	 Widening EB Froom Ranch Way to add a dedicated right turn pocket would improve this condition to an acceptable level. Extension of the westbound right turn pocket to 110' would improve this condition to an acceptable level. Approximately 12' of additional right-of-way from the adjacent vacant parcel may be required along with reconstruction of the frontage road.
Intersection #10 Los Osos Valley Road/Auto Park Way	Auto	The Project added traffic increases the delay at this intersection to be unacceptable LOS E during the PM peak hour.	Construction of the Prado Overcrossing would improve this condition to an acceptable level. Signalization will also provide acceptable operations.
Intersection #11 Los Osos Valley Road/Calle Joaquin	Auto (Queue)	Addition of project traffic would result in a new impact to the southbound left queue in the AM and PM peak hours.	Recently constructed LOVR improvements improve this condition to an acceptable level.
Intersection #12 Los Osos Valley Road/U.S. 101 SB Ramps	Auto (Queue)	Addition of project traffic would: •Exacerbate the westbound left queue in the AM and PM peak hours. •Exacerbate the southbound through queue in the AM peak hour. •Exacerbate the southbound right queue in the AM and PM peak hours.	Extension of the off ramp left turn pocket to 320' would improve this condition to an acceptable level. Extension of the southbound through storage and southbound right turn pocket is not feasible due to intersection spacing. However, traffic queues between the two intersections are managed through the signal coordination of the Calle Joaquin and 101 SB intersections.

TABLE 41: EXISTING PLUS PROJECT MITIGATION MEASURES SUMMARY

Existing Plus Project Significant Impact Location	Mode	Impact	Mitigation Measure
Intersection #13 Los Osos Valley Road/U.S. 101 NB Ramps	Auto (Queue)	Addition of project traffic would: •Exacerbate the southbound through queue in the AM peak hour. •Exacerbate the southbound right queue in the AM and PM peak hours.	Recently constructed LOVR improvements improve the southbound through queue impact to an acceptable level, and construction of the Prado Overcrossing would improve this condition to an acceptable level.
Intersection #14 Los Osos Valley Road/S. Higuera Street	Auto (Queue)	Addition of project traffic would exacerbate the eastbound right queue in the AM and PM peak hours.	Extension of the eastbound right turn pocket to 180' would improve this condition to an acceptable level, but may require additional right-ofway.
Intersection #16 S. Higuera St/Tank Farm Rd	Auto (Queue)	Addition of project traffic would exacerbate the northbound right queue in the AM peak hour.	Extension of the northbound right turn pocket to 160' would improve this condition to an acceptable level, but may require additional right-ofway.
Intersection #18 S. Higuera St/Prado Rd	Auto (Queue)	Addition of project traffic would: •Exacerbate the northbound left queue in the AM peak hour.	Widening Prado Road to add a second NB left would improve this condition to acceptable levels.
Intersection #25 Dalidio Dr/SC Project Dwy	Auto	The Project added traffic increases the delay at this intersection to be unacceptable LOS E during the PM peak hour.	Install a Multi-lane modern roundabout
Segment #1: Madonna Rd WB - Oceanaire Dr to Los Osos Valley Rd	Auto	The Project added traffic increases the delay on this segment to be unacceptable LOS E during the PM peak hour.	Construction of the Prado Overcrossing would improve this condition to an acceptable level.
Segment #2: Madonna Rd EB - Oceanaire Dr to Dalidio Dr	Auto	The Project added traffic increases the delay on this segment to be unacceptable LOS E during the AM and	Improvements to Intersection #3 (Madonna Rd/Dalidio Dr) mitigates the impact to this segment.

TABLE 41: EXISTING PLUS PROJECT MITIGATION MEASURES SUMMARY

Existing Plus Project Significant Impact Location	Mode	Impact	Mitigation Measure
		PM peak hours.	
Segment #3: Madonna Rd WB - El Mercado to Dalildio Dr	Transit	The Project added traffic increases the delay on this segment to be unacceptable LOS E during the AM peak hour.	Decrease Transit Route 5 headway from 30 minutes to 25 minutes.
Segment #6: Madonna Rd EB - US 101 NB Ramps to Higuera St	Transit	The Project added traffic increases the delay on this segment to be unacceptable LOS E during the PM peak hour.	Decrease Transit Route 5 headway from 30 minutes to 25 minutes.
Segment #13: Los Osos Valley Rd NB - Froom Ranch Way to Madonna Rd	Auto	The Project added traffic increases the delay on this segment to be unacceptable LOS E during the PM peak hour.	Construction of the Prado Overcrossing would improve this condition to an acceptable level.

Table 42 presents the mitigated intersection LOS operations assuming the above mitigation measures to be in place. Table 43 presents the mitigated roadway segment LOS operations assuming the above mitigation measures to be in place.

TABLE 42: EXISTING PLUS PROJECT – MITIGATED INTERSECTION LOS

Intersection #	3	3		5	3	3	9		1	0	1:	5	2	25
Name	Madonn Dalidio		US 10	a Road/ 01 SB mps	Higuera South		Los Oso Road/ Ranch	Froom	Los Oso Road/ Park	Auto	S. Hig Stre Suburba	eet/	SC F	Drive/ Project eway
Mode	AU	ТО	AU	ТО	AU	ТО	BI	(E	AU'	ТО	PE	D	AU	ITO
Scenario	v/c	LOS	v/c	LOS	v/c	LOS	Score	LOS	v/c	LOS	v/c	LOS	v/c	LOS
AM PEAK HOUR							EB				NB			
Existing	-	Α	1.31	D	-	С	3.38	С	-	В	2.98	С	-	-
Existing Plus Project	-	В	0	С	-	С	3.40	С	-	В	2.99	С	-	Α
Significant Impact?	N	О	Ν	lo	N	lo	N	0	N	0	N	0	١	l o
PM PEAK HOUR							EB				NB			
Existing	2.11	Е	-	С	1.31	Е	4.39	Е	-	D	3.60	D	_	-
Existing Plus Project	-	С	-	С	_	С	4.42	Е	0.39	Е	3.61	D	-	В
Significant Impact?	N	o	Ν	lo	N	lo	N	0	Ye	es	N	0	١	No.

Note: V/C is based on worst movement

TABLE 43: EXISTING PLUS PROJECT – MITIGATED SEGMENT LOS

Existing Plus Pr	oject Multimodal Seg	ment LOS				Au	to Mode					Pedestr	rian Mode					Bicyc	le Mode					Tran	sit Mode		
AM PEAK HOUR	R				Exist	ing	Existing Pla	us Project			Existi	ng	Existing Plus	s Project	3		Existi	ng	Existing Plus	s Project	•		Existing		Existing Plus P	roject	
				Auto LOS	Travel		Travel		Significa	Ped LOS					Significa	Bike LOS					Significa	Transit LOS					
				Threshold	Speed		Speed		nt	Threshold	Segment		Segment		nt	Threshold	Segment		Segment		nt						Significant
ID Roadway	From	То	Direction		(mph)	LOS	(mph)	LOS	Impact?		Score	LOS	Score	LOS	Impact?		Score	LOS	Score	LOS	Impact?	Threshold	Segment Score	LOS	Segment Score	LOS	Impact?
2 Madonna Rd	Oceanaire Dr	Dalidio	EB	D	27.1	С	21.3	С	No	С	3.80	D	3.88	D	No	D	3.57	D	3.58	D	No	D	4.70	E	4.74	E	No
3 Madonna Rd	El Mercado	Dalidio Dr	WB	D	21.2	С	18.7	С	No	С	3.52	D	3.61	D	No	D	3.27	С	3.30	С	No	D	4.24	D	4.20	D	No
20 Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	D	5.5	F	28.6	В	No	С	3.04	С	3.13	С	No	D	3.77	D	3.32	С	No	D	4.26	E	4.20	D	No

Note: Segment 20 transit is southbound for routes 4 and 5; Segment 21 pedestrian and bicycle service will be further evaluated using off-street facilities methodologies.

Existing Plus Pro	oject Multimodal Seg	ment LOS				Au	to Mode				ı	Pedest	trian Mode					Bicyc	le Mode					Trans	sit Mode		
PM PEAK HOUR					Existi	ing	Existing Plu	s Project			Existin	ng	Existing Plu	s Project			Exist	ing	Existing Plu	ıs Project			Existing		Existing Plus Pi	roject	
				Auto LOS Threshold	Speed		Travel Speed		Significa nt	Threshold	Segment		Segment		nt		Segment		Segment		Significa nt	LOS Threshold					Significant
ID Roadway	From	То	Direction		(mph)	LOS	(mph)	LOS	Impact?		Score	LOS	Score	LOS	Impact?		Score	LOS	Score	LOS	Impact?		Segment Score	LOS	Segment Score	LOS	Impact?
1 Madonna Rd	Oceanaire Dr	LOVR	WB	D	12.7	E	15.9	E	No	С	3.79	D	3.82	D	No	D	3.93	D	3.97	D	No	D	4.28	E	4.32	E	No
2 Madonna Rd	Oceanaire Dr	Dalidio	EB	D	18.9	D	20.8	С	No	С	3.89	D	3.95	D	No	D	3.43	С	3.45	С	No	D	4.51	Е	4.53	Е	No
6 Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	13.3	Е	12.4	E	No	С	3.72	D	3.77	D	No	D	3.53	D	3.59	D	No	D	4.18	D	4.24	D	No
13 Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	16.9	D	16.8	D	No	С	4.04	F	4.04	F	No	D	3.46	С	3.46	С	No	D	4.38	E	4.38	E	No
20 Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	D	0.4	F	18.0	D	No	С	3.35	С	3.46	С	No	D	4.21	D	3.42	С	No	D	4.09	D	4.03	D	No

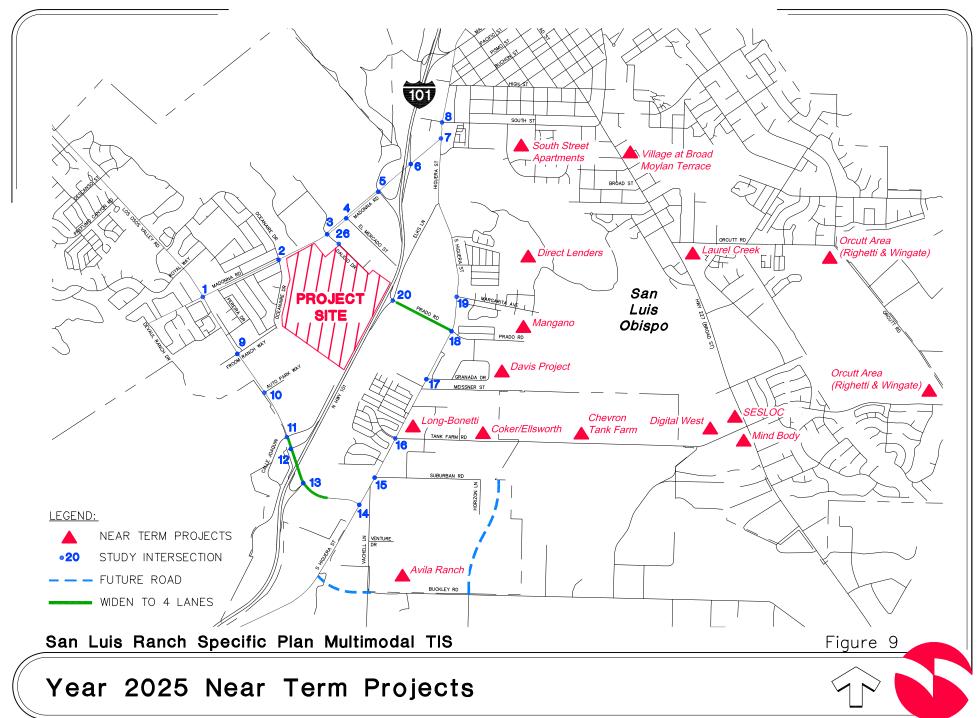
Note: Segment 20 transit is southbound for routes 4 and 5; Segment 21 pedestrian and bicycle service will be further evaluated using off-street facilities methodologies.

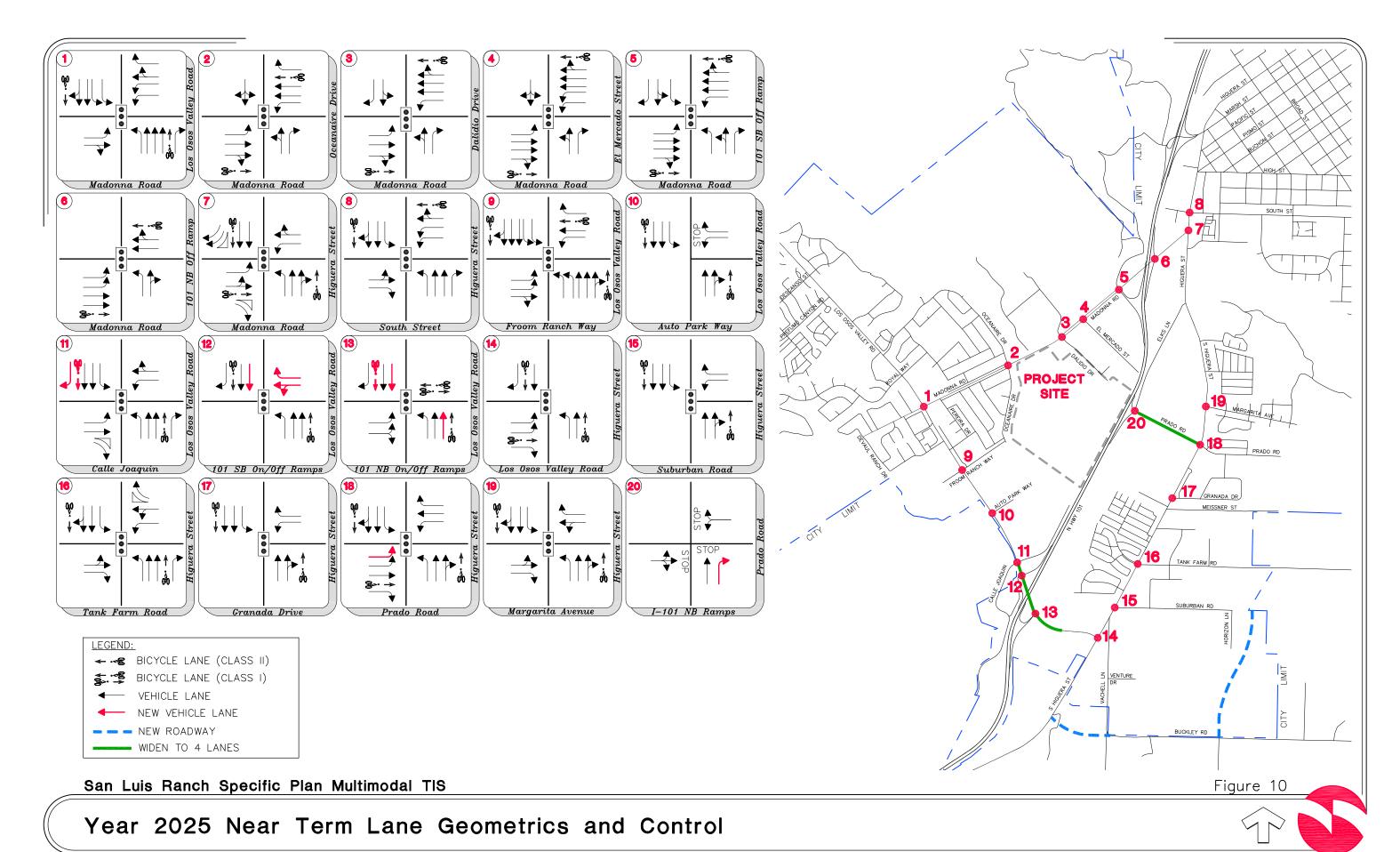
Near Term (Year 2025) Conditions

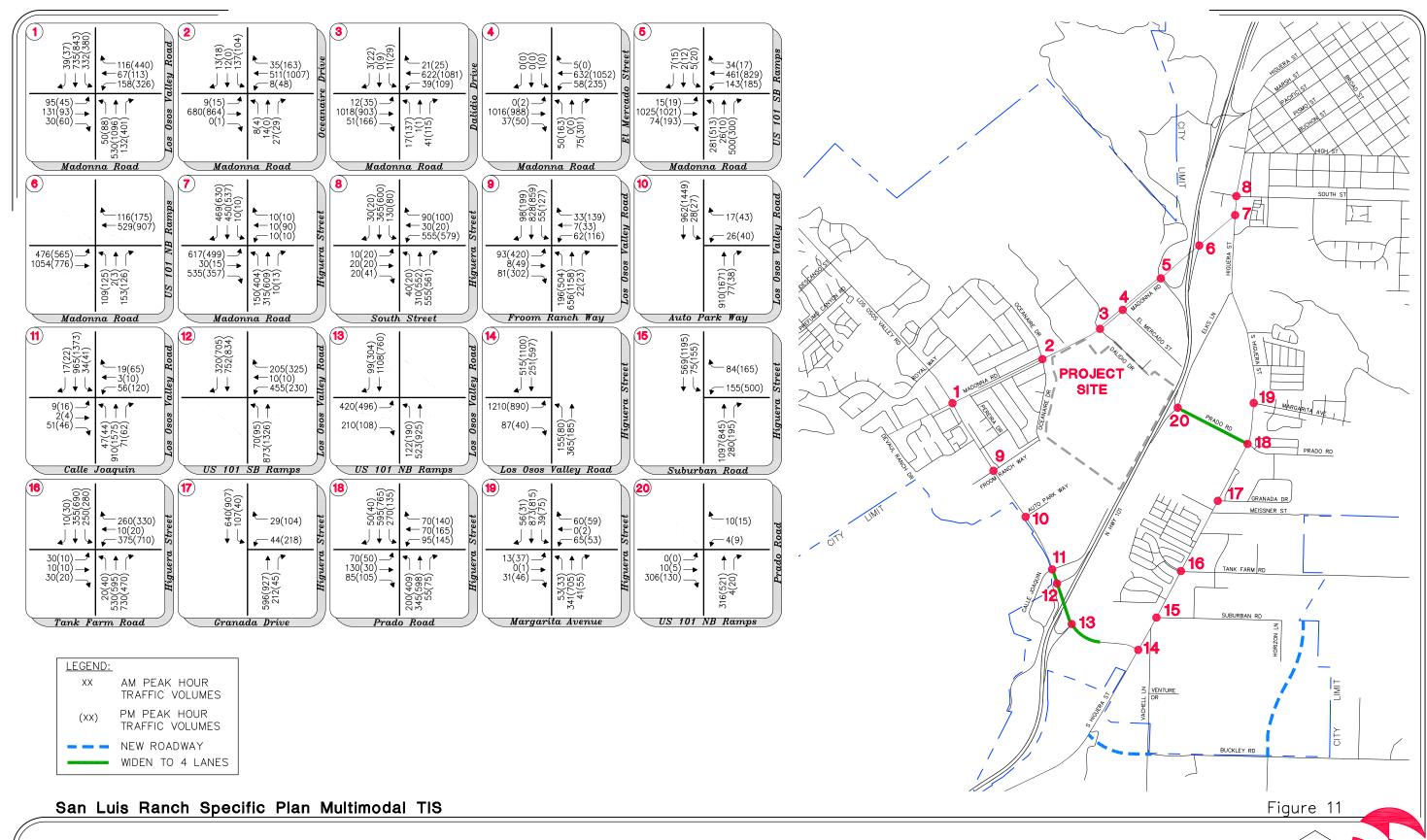
The Near Term conditions is a scenario in which the City's approved, pending and potential land development projects are assumed to be in place. Based on direction from the City, a volume growth increment for all travel modes have been developed for the 2025 conditions. Vehicular trips are determined utilizing the *Avila Ranch* study completed by Central Coast Transportation Consulting. The volume growth between the Avila Ranch's *Existing* volumes and the *Near Term Plus Project No Business Park* volumes were utilized as a baseline to estimate a growth increment which was added to the Existing volumes shown in Figure 3. Roadway improvements assumed to be in place for the Near Term conditions are as follows:

- Los Osos Valley Road interchange improvements and widening to 4 Lanes between Calle Joaquin and S. Higuera Street with Class II Bike Lanes
- Prado Road widening to 4 lanes between US 101 and S. Higuera Street with Class II Bike Lanes and an additional westbound left turn lane at S. Higuera Street
- Horizon Lane extension between Avila Ranch and Suburban Drive
- Southbound left turn pocket at Prado Road/S. Higuera Street is extended 250' with addition of pedestrian countdown heads with audible/tactile pushbuttons.
- Buckley Road extension to S. Higuera Street

Figure 9 presents the Year 2025 Near Term Approved and Pending Projects that are assumed to be in place for the Near Term conditions analysis scenarios. Figure 10 presents the Near Term lane geometrics and control assuming the above roadway improvements are in place. Figure 11 presents the Year 2025 Near Term peak hour traffic volumes assuming the above roadway improvements are in place, and with buildout of the Near Term Approved and Pending Projects.







Near Term No Project Conditions Analysis

The Near Term conditions multimodal analysis for the study intersections and segments are presented below.

Near Term No Project Conditions Intersection Analysis

Table 44 provides a summary of the Near Term conditions vehicular AM and PM peak hour intersection delay and LOS. Table 45 provides a summary of the Near Term pedestrian AM and PM peak hour conditions at the study intersections. Table 46 provides a summary of the Near Term bicycle AM and PM peak hour conditions at the study intersections. Table 47 provides a summary of the Near Term conditions queuing analysis.

TABLE 44: NEAR TERM NO PROJECT CONDITIONS INTERSECTION LEVEL OF SERVICE: AUTOMOBILE ANALYSIS

v/c³ Delay LOS 51.8 D 17.7 B 43.1 D 17.5 B 25.0 C
51.8 D 17.7 B 43.1 D 17.5 B
17.7 B 43.1 D 17.5 B
17.5 B
25.0 C
21.1 C
38.5 D
1.28 65.8 E
36.4 D
0.52 53.6 F
6.3 A
20.0 B
21.0 C
29.3 C
19.9 B
25.1 C
11.5 B
27.8 C
14.6 B
12.1 B

^{1.} AWSC = All Way Stop Control; TWSC = Two Way Stop Control; RNDBT = Roundabout

^{2.} LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC,

^{3.} Volume to Capacity Ratio (v/c) is for worst movement delay, for unacceptable LOS only

TABLE 45:
NEAR TERM CONDITIONS INTERSECTION LEVEL OF SERVICE: PEDESTRIAN ANALYSIS

				AM Peak	Hour	PM Peak	Hour
			Target	Ped. Crosswalk		Ped. Crosswalk	
ŧ	Intersection	Approach	LOS	Score	LOS	Score	LOS
		EB	C	2.11	B	2.13	B
		WB	Ċ	2.88	Ċ	3.29	Ċ
	Madonna Road/Los Osos Valley Road	NB	С	3.11	С	3.51	D
		SB	С	3.20	C 3.29 C 3.51 D C 3.51 D C 3.29 C 3.51 C D C 3.29 C C 3.34 C A 2.00 A A 1.85 A 2.00 A B		
		EB	С	2.70			
		WB	Č	3.08			
	Madonna Road/Oceanaire Drive	NB	Ċ	1.99			
		SB	C	1.81			
_		EB	C	2.96			
		WB	C	2.98			
	Madonna Road/Dalidio Drive	NB	Č	2.05			
			C				
-		SB	C	1.96			
		EB WB	C	n/a 3.05			
	Madonna Road/El Mercado	NB	C	2.28			
_		SB	С	1.71			
		EB	С	3.00			
	Madonna Road/US 101 SB	WB	С	n/a			
	Ramps/Madonna Inn	NB	С	2.78			
		SB	С	2.17	В		В
		EB	С	n/a			
	Madonna Road/US 101 NB Ramps	WB	С	2.88			
		NB	С	2.01	В		В
		SB	С	n/a			
٦		EB	С	2.93			
.	Madonna Road/Higuora Stroot	WB	С	2.01			
'	Madonna Road/Higuera Street	NB	С	2.79	С	2.91	С
		SB	С	n/a		n/a	
		EB	С	2.03	В	2.06	В
		WB	С	2.78	С	2.80	С
	Higuera Street/South Street	NB	С	n/a	-	n/a	-
		SB	С	2.51	В	2.60	В
		EB	С	2.51			
	Los Osos Valley Road/Froom Ranch	WB	Č	2.39			
١	Way	NB	C	n/a			
	way	SB	С	3.10	C		C
_		EB	C	3.10		3.33	
		WB	C				
0	Los Osos Valley Road/Auto Park Way	NB	C	n/a		n/a	
		SB	C				
_		EB	C	2.47	- D	2.20	- D
		WB	C	2.47 2.07			
1	Los Osos Valley Road/Calle Joaquin	NB	C	3.02			
		SB	C	I			
			_	3.00			
		EB	С	1.85			
2	Los Osos Valley Road/US 101 SB	WB	С	2.14	В	2.08	В
	Ramps	NB	С	n/a	-	n/a	-
		SB	С	n/a	-	n/a	-
	Los Osos Valley Road/US 101 NB	EB	С	2.36	В	2.36	В
3	Ramps	NB	С	2.78	С	2.79	С
		SB	С	n/a	-	n/a	-
٦		EB	С	2.99	С	2.91	С
4	S. Higuera Street/Los Osos Valley Road	NB	С	2.38	В	2.41	В
	,	SB	С	n/a	-	n/a	-
٦		WB	C	2.33	В	2.57	В
5	S. Higuera Street/Suburban Drive	NB	č	3.23	Č	3.96	Ď
		SB	С	2.88	С	3.02	С
		EB	С	2.03	В	2.02	В
٦	S. Higuera Street/Tank Farm Road	WB	С	3.01	С	3.19	С
1	o. mgdera ouees rank rann Noau	NB	С	3.46	C	3.38	C
		SB	С	2.77	С	2.91	С
		WB	С	2.11	В	2.18	В
7	S. Higuera Street/Granada Drive	NB	С	n/a	-	n/a	-
		SB	С	2.71	В	2.89	С
٦		EB	С	2.66	В	2.72	В
١,	S. Higuera Stroot/Brada Bood	WB	С	2.40	В	2.48	В
١,	S. Higuera Street/Prado Road	NB	С	2.90	С	3.19	С
		SB	C	2.86	Č	2.92	Ċ
_		EB	С	2.28	В	2.09	В
		WB	C	2.13	В	2.20	В
	S. Higuera Street/Margarita Avenue	NB	C	2.85	C	2.93	C
9			C	2.76	C	2.90	C
9		SB					U
9		SB		2.10			
9		EB	С	2.70	-		-
	Prado Road/US 101 NB Ramps			n/a	-	n/a	-

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. HCM 2010 Methodologies for the pedestrian mode at two-way stop-controlled intersections is limited to the uncontrolled crossing. No methodology exists for evaluating pedestrian performance for the stop controlled approach (cross-street). However, it is reasoned that this type of control has negligible influence on pedestrian service along the segment.

TABLE 46:
NEAR TERM CONDITIONS INTERSECTION LEVEL OF SERVICE: BICYCLE ANALYSIS

				AM Pea		PM Pea	k Hour
			Target	Bicycle LOS		Bicycle LOS	
#	Intersection	Approach	LOS	Score	LOS	Score	LOS
		EB	D	3.28	С	3.17	С
1	Madonna Road/Los Osos Valley Road	WB	D	3.36	C	4.25	D
		NB	D	1.69	A	2.17	В
		SB	D	2.69	В	2.68	В
		EB	D	2.71	В	2.89	С
2	Madonna Road/Oceanaire Drive	WB	D	1.15	Α	1.75	Α
-	INACOTTIC PROGRAMME BITTE	NB	D	2.74	В	2.72	В
		SB	D	2.22	В	2.15	В
		EB	D	2.14	В	2.07	В
_		WB	D	1.54	Α	1.80	Α
3	Madonna Road/Dalidio Drive	NB	D	3.00	С	3.32	С
		SB	D	2.84	C	2.92	C
		EB	D	1.77	A	1.70	A
		WB	D	1.73	A	2.02	В
4	Madonna Road/El Mercado	NB	D	3.26		1	
					С	3.81	D
_		SB	D	3.03	C	3.03	C
		EB	D	1.96	Α	1.97	Α
5	Madonna Road/US 101 SB	WB	D	1.66	Α	1.86	Α
	Ramps/Madonna Inn	NB	D	n/a	-	n/a	-
		SB	D	2.90	С	2.96	С
		EB	D	2.62	В	2.25	В
6	Madonna Road/US 101 NB Ramps	WB	D	1.67	Α	1.96	Α
		NB	D	n/a	-	n/a	-
		EB	D	3.36	С	2.74	В
	L	WB	D	2.46	В	2.60	В
7	Madonna Road/Higuera Street	NB	D	1.80	A	2.27	В
		SB	D	2.29	В		В
		EB	D	2.29	В	2.48 2.78	С
				1			
В	Higuera Street/South Street	WB	D	2.75	В	2.83	С
	-	NB	D	3.01	С	3.24	С
		SB	D	1.58	A	1.75	Α
		EB	D	3.42	С	4.42	E
9	Los Osos Valley Road/Froom Ranch	WB	D	1.86	Α	2.50	В
•	Way	NB	D	1.84	Α	2.28	В
		SB	D	1.78	Α	1.87	Α
		EB	D		-		-
		WB	D		_		_
0	Los Osos Valley Road/Auto Park Way	NB	D	n/a	_	n/a	_
		SB	D				
		EB	D	2.99	C	2.99	C
		WB	D	3.08	C	3.27	C
1	Los Osos Valley Road/Calle Joaquin		D			1	
		NB		1.54	A	2.02	В
		SB	D	0.50	A	0.79	A
	Los Osos Valley Road/US 101 SB	WB	D	n/a	-	n/a	-
2	Ramps	NB	D	2.86	С	3.20	С
	rtapo	SB	D	1.66	Α	1.98	Α
	Lee Oses Valley Bood/US 101 NB	EB	D	n/a	-	n/a	-
3	Los Osos Valley Road/US 101 NB Ramps	NB	D	1.99	Α	2.35	В
	Italiips	SB	D	2.90	С	2.74	В
		EB	D	2.18	В	1.85	Α
4	S. Higuera Street/Los Osos Valley Road	NB	D	2.20	В	1.74	Α
		SB	D	2.61	В	4.21	D
	1	WB	D	1.08	A	1.81	A
5	S Higuera Street/Suburban Driva	NB	D	2.43	В	2.09	В
J	S. Higuera Street/Suburban Drive						
		SB	D	1.81	A	2.41	B
		EB	D	2.72	В	2.67	В
6	S. Higuera Street/Tank Farm Road	WB	D	2.64	В	3.32	С
		NB	D	2.39	В	2.20	В
		SB	D	1.78	A	2.10	В
		WB	D	2.63	В	3.07	С
7	S. Higuera Street/Granada Drive	NB	D	1.90	Α	2.05	В
		SB	D	1.99	Α	2.17	В
		EB	D	2.48	В	2.30	В
_	O Historia Otro at/Day 1 D	WB	D	2.93	C	3.31	C
ಠ	S. Higuera Street/Prado Road	NB	D	1.76	A	2.18	В
		SB	D	2.04	В	2.06	В
		EB	D	2.49	В	2.57	В
				1			
9	S. Higuera Street/Margarita Avenue	WB	D	2.73	В	2.71	В
	_	NB	D	1.68	A	1.99	Α
_		SB	D	2.23	В	2.20	В
		EB	D	1	-		-
0	Prado Road/US 101 NB Ramps	WB	D	n/a	-	n/a	-
U	I lado Noadroo To FNB Namps	NB	D	ı#a	-	1"a	-
	1	SB	D	1		1	

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. No methodology exists for evaluating bicycle performance at two-way stop-controlled intersections. However, it is reasoned that this type of control has negligible influence on bicycle service along the segment for stop controll on the cross-street.

TABLE 47:
NEAR TERM CONDITIONS 95TH PERCENTILE QUEUING ANALYSIS

Interse	ction			Total	95 th Per Queue/I	
ID	Location	Movement	No. Lanes	Storage (ft) ¹	AM Peak Hour	PM Peak Hour
		Westbound Through/Right	1	340	99	3822
1	Madonna Road/Los	Westbound Right	1	300	60	350
l '	Osos Valley Road	Northbound Left	1	200	70	253
		Northbound Right	1	175	119	248
2	Madonna	Westbound Right	1	100	36	139
5	Madonna Road/US 101	Westbound Left	1	260	160	322
7	Madonna Road/Higuera	Eastbound Right	1	150	221	192
,	Street	Northbound Left	1	160	153	361
		Westbound Left	2	240	303	287
8	Higuera Street/South	Northbound Left	1	60	103	77
8	Street	Northbound Right	1	60	135	143
		Southbound Left	1	70	115	118
9	Los Osos Valley	Westbound Right	1	50	50	81
	Los Osos Valley	Westbound Left/Through	1	180	256	187
12	Road/US 101 SB	Southbound Through	1	240	275	284
	Ramps	Southbound Right	1	125	188	221
		Eastbound Left/Right	1	625	325	288
	Los Osos Valley	Northbound Left	1	395	134	181
13	Road/US 101 NB	Northbound Through	1	-	119	164
	Ramps	Southbound Through	1	865	542	257
		Southbound Right	1	130	215	187
14	S. Higuera Street/Los	Eastbound Right	1	90	202	154
45	S. Higuera	Westbound Right	1	170	69	249
15	Street/Suburban Drive	Southbound Left	1	200	158	272
10	S. Higuera Street/Tank	Northbound Right	1	100	197	182
16	Farm Road	Southbound Left	1	165	210	231
17	S. Higuera	Southbound Left	1	80	94	77
40	S. Higuera Street/Prado	Westbound Left	1	105	110	140
18	Road	Northbound Left	1	100	158	173
10	S. Higuera	Northbound Left	1	60	75	57
19	Street/Margarita Avenue	Southbound Left	1	60	68	91

Notes: 1. Bolded entries indicate queues exceed available storage

- 2. Storage Length of " " represents a lane which exceeds 900 feet, usually a through lane.
- 3. For Movements with more than one lane, the maximum of the 95th percentile queue is reported.
- 4. * Represents storage lengths for one lane; second lane is a left or right trap lane.

As shown in Table 44 above, the intersections of Madonna Road/US 101 SB Ramps/Madonna Inn, Higuera Street/South Street, Los Osos Valley Road/Auto Park Way, and S. Higuera Street/Tank Farm Road are projected to operate at unacceptable conditions during Near Term conditions. The northbound and southbound approaches experience long delays. Pedestrian and bicycle analysis shows acceptable conditions at the study intersections, except for the intersections of Los Osos Valley Road/Madonna Road and Los Osos Valley Road/Froom Ranch Way, respectively. Queuing analysis results show numerous locations in which the 95th percentile queue exceeds the available storage capacity during Near Term conditions.

Near Term No Project Conditions Segment Analysis

Table 48 provides a summary of the Near Term vehicular AM and PM peak hour conditions for the study segments. Table 49 provides a summary of the Near Term pedestrian AM and PM peak hour conditions for the study segments. Table 50 provides a summary of the Near Term bicycle AM and PM peak hour conditions for the study segments. Table 51 provides a summary of the Near Term transit AM and PM peak hour conditions for the study segments. Table 52 provides a summary of the Near Term freeway segments analysis for AM and PM peak hour conditions for the study segments along US 101.

TABLE 48:
NEAR TERM CONDITIONS SEGMENT LEVEL OF SERVICE: AUTOMOBILE ANALYSIS

AUTO SEGMENT LOS							AM	PEAK			PM P	EAK	
					1.00		Base Free-			Travel	Base Free-	Travel	
					LOS	Travel Speed	Flow Speed	Travel Speed/		Speed	Flow Speed	Speed/	
ID	Roadway	From	То	Direction	Threshold	(mph)	BFFS (mph)	BFFS (%)	LOS	(mph)	BFFS (mph)	BFFS (%)	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	С	22.1	42.1	52%	С	11.8	42.1	28%	F
	Madonna Rd	LOVR	Oceanaire Dr	EB	С	27.5	42.1	65%	С	29.3	42.1	70%	В
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	С	23.0	40.8	56%	С	24.4	40.7	60%	С
	Madonna Rd	Oceanaire Dr	Dalidio	EB	С	27.1	40.7	67%	С	18.7	40.8	46%	D
3	Madonna Rd	El Mercado	Dalidio Dr	WB	С	19.8	34.1	58%	С	13.9	34.8	40%	E
	Madonna Rd	Dalidio Dr	El Mercado	EB	С	26.5	38.2	69%	В	12.5	34.6	36%	E
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	С	31.1	37.9	82%	В	22.3	37.3	60%	С
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	С	21.3	37.8	56%	С	18.4	37.7	49%	D
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	С	28.6	37.8	76%	В	22.7	37.8	60%	С
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	С	33.0	37.8	87%	Α	33.6	37.8	89%	Α
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	С	11.2	37.2	30%	E	12.9	37.2	35%	E
	Madonna Rd	US 101 NB Ramps	Higuera St	EB	С	13.1	37.2	35%	E	10.4	37.2	28%	F
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	С	34.0	44.5	76%	В	33.2	44.5	75%	В
	S. Higuera St	Margarita Ave	Madonna Rd	NB	С	33.0	44.8	74%	В	33.3	44.8	74%	В
8	S. Higuera St	Margarita Ave	Prado Rd	SB	С	16.2	38.9	42%	D	11.6	38.9	30%	F
	S. Higuera St	Prado Rd	Margarita Ave	NB	С	22.7	38.9	58%	С	19.1	38.9	49%	D
9	S. Higuera St	Prado Rd	Granada Dr	SB	С	33.5	41.8	80%	В	29.3	41.8	70%	В
	S. Higuera St	Granada Dr	Prado Rd	NB	С	21.6	41.9	52%	С	24.1	41.9	57%	С
10	S. Higuera St	Granada Dr	Tank Farm Road	SB	С	42.3	41.6	102%	Α	24.7	42.6	58%	С
	S. Higuera St	Tank Farm Road	Granada Dr	NB	С	29.7	41.6	71%	В	26.8	42.6	63%	С
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	С	27.1	42.4	64%	С	21.6	41.2	52%	С
	S. Higuera St	Suburban Drive	Tank Farm Road	NB	С	17.8	42.5	42%	D	16.3	41.3	39%	E
12	S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	С	20.4	42.1	49%	D	13.5	39.1	34%	E
	S. Higuera St	Los Osos Valley Road	d Suburban Drive	NB	С	23.8	42.0	57%	С	19.1	39.0	49%	D
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	С	24.0	41.9	57%	С	16.9	41.8	41%	D
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	С	20.9	41.8	50%	С	15.1	41.8	36%	E
14	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	С	29.1	43.0	68%	В	30.5	43.0	71%	В
	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	С	30.0	43.2	69%	В	22.9	43.2	53%	С
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	С	6.1	32.1	19%	F	13.2	32.1	41%	D
	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	С	16.2	31.1	52%	С	14.1	31.1	45%	D
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	С	23.5	37.7	62%	С	17.5	37.7	46%	D
	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	С	20.3	37.4	54%	С	32.2	37.4	86%	Α
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	С	17.5	39.4	45%	D	15.8	39.4	40%	D
	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	С	24.5	39.2	63%	С	27.0	39.2	69%	В
18	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	С	19.7	39.1	50%	С	23.1	39.1	59%	С
	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	С	23.4	39.0	60%	С	21.7	39.0	56%	С
19	Froom Ranch W	End	Los Osos Valley	WB	С	18.0	37.7	48%	D	13.0	37.9	34%	E
	Froom Ranch Wa	Los Osos Valley	End	EB	С	35.5	38.1	93%	Α	34.6	37.4	92%	Α
20	Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	С	21.7	31.2	70%	В	21.6	31.2	69%	В
	Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	С	6.1	31.1	20%	F	0.6	31.1	2%	F

TABLE 49:
NEAR TERM CONDITIONS SEGMENT LEVEL OF SERVICE: PEDESTRIAN ANALYSIS

PEDESTRIAN SEGM	ENT LOS					AM P	EAK	PM PE	AK
				LOS	Average Ped.	SEGMENT		SEGMENT	
ID Roadway	From	То	Direction	Threshold	Space (ft²/p)	SCORE	LOS	SCORE	LOS
1 Madonna Rd	Oceanaire Dr	LOVR	WB	С	6090	3.52	D	3.86	D
Madonna Rd	LOVR	Oceanaire Dr	EB	С	17482	3.93	D	4.06	D
2 Madonna Rd	Dalidio	Oceanaire Dr	WB	С	84000	3.62	D	3.97	D
Madonna Rd	Oceanaire Dr	Dalidio	EB	С	26250	3.80	D	3.88	D
3 Madonna Rd	El Mercado	Dalidio Dr	WB	С	37450	3.53	D	3.86	D
Madonna Rd	Dalidio Dr	El Mercado	EB	С	52920	3.68	D	3.75	D
4 Madonna Rd	US 101 SB Ramps	El Mercado	WB	С	26250	3.62	D	3.79	D
Madonna Rd	El Mercado	US 101 SB Ramps	EB	С	27915	3.70	D	3.77	D
5 Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	С	No Peds	3.69	D	3.86	F
Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	С	No Peds	4.04	D	3.95	D
6 Madonna Rd	Higuera St	US 101 NB Ramps	WB	С	25200	3.63	D	3.80	D
Madonna Rd	US 101 NB Ramps	Higuera St	EB	c	19838	3.84	D	3.76	D
7 S. Higuera St	Madonna Rd	Margarita Ave	SB	c	23247	3.86	D	3.88	D
S. Higuera St	Margarita Ave	Madonna Rd	NB	С	5398	3.67	D	3.90	D
8 S. Higuera St	Margarita Ave	Prado Rd	SB	С	40979	3.71	D	3.73	D
S. Higuera St	Prado Rd	Margarita Ave	NB	С	21700	3.55	D	3.71	D
9 S. Higuera St	Prado Rd	Granada Dr	SB	С	9292	3.65	D	3.77	D
S. Higuera St	Granada Dr	Prado Rd	NB	С	8400	3.26	С	3.52	D
S. Higuera St	Granada Dr	Tank Farm Road	SB	С	46305	3.62	D	3.80	D
S. Higuera St	Tank Farm Road	Granada Dr	NB	c	49140	3.26	C	3.36	C
S. Higuera St	Tank Farm Road	Suburban Drive	SB	c	12600	3.66	D	3.96	D
S. Higuera St	Suburban Drive	Tank Farm Road	NB	c	31500	3.66	D	3.56	D
S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	С	39312	3.64	D	4.01	D
S. Higuera St	Los Osos Valley Road	•	NB	c	43533	4.00	D	4.05	D
Los Osos Valle	/ Madonna Rd	Froom Ranch Way	SB	С	21833	3.86	D	3.99	D
Los Osos Valle	/ Froom Ranch Way	Madonna Rd	NB	С	0	3.74	F	4.19	F
Los Osos Valle	/ Froom Ranch Way	Calle Joaquin	SB	С	27300	3.84	D	4.05	D
Los Osos Valle	, Calle Joaquin	Froom Ranch Way	NB	С	22050	3.75	D	4.09	D
Los Osos Valle	' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' '	US 101 SB Ramps	SB	С	44100	3.69	D	3.70	D
	US 101 SB Ramps	Calle Joaquin	NB	c	63000	3.66	D	4.01	D
	/ US 101 SB Ramps	US 101 NB Ramps	SB	С	No Peds	3.93	D	3.91	D
	US 101 NB Ramps	US 101 SB Ramps	NB	c	53928	3.82	D	3.27	c
	/ US 101 NB Ramps	S. Higuera St	EB	C	46575	3.94	D	3.78	D
	/ S. Higuera St	US 101 NB Ramps	WB	С	1680	3.88	D	4.27	E
18 Prado Rd	S. Higuera St	US 101 NB Ramps	WB	С	56133	2.76	С	2.84	С
Prado Rd	US 101 NB Ramps	S. Higuera St	EB	С	3019	3.44	C	3.42	C
19 Froom Ranch V	'	Los Osos Valley	WB	c	No Peds	3.32	С	3.53	D
	Va Los Osos Valley	End	EB	c	75600	1.67	A	1.81	A
20 Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	C	56700	1.46	A	1.49	A
Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	c	73710	3.04	C	3.38	C
Dallulo Di	1100111 Naticii way	ινιαυσιπα Νυ	IND		/3/10	3.04		3.30	

TABLE 50: NEAR TERM CONDITIONS SEGMENT LEVEL OF SERVICE: BICYCLE ANALYSIS

BIC	YCLE SEGMENT LOS					AM PE	AK	PM PE	AK
					LOS	SEGMENT		SEGMENT	
ID	Roadway	From	То	Direction	Threshold	SCORE	LOS	SCORE	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	D	3.60	D	4.13	D
	Madonna Rd	LOVR	Oceanaire Dr	EB	D	3.73	D	3.78	D
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	D	3.14	С	3.23	С
	Madonna Rd	Oceanaire Dr	Dalidio	EB	D	3.56	D	3.43	С
3	Madonna Rd	El Mercado	Dalidio Dr	WB	D	3.29	С	3.21	С
	Madonna Rd	Dalidio Dr	El Mercado	EB	D	3.38	С	3.39	С
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	3.96	D	4.35	Е
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	3.61	D	3.62	D
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	3.31	С	3.36	С
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	3.37	C	3.31	C
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	3.50	D	3.56	
_	Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	3.69	D	3.52	D
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	D	3.90		3.90	D
•	S. Higuera St	Margarita Ave	Madonna Rd	NB	D	4.09	D	4.18	D
8	S. Higuera St	Margarita Ave	Prado Rd	SB	D	3.67	D	3.67	D
Ū	S. Higuera St	Prado Rd	Margarita Ave	NB	D	3.90	D	3.96	D
9	S. Higuera St	Prado Rd	Granada Dr	SB	D	3.88	D	3.91	D
,	S. Higuera St	Granada Dr	Prado Rd	NB	D	3.45	C	3.53	D
10	S. Higuera St	Granada Dr	Tank Farm Road	SB	D	4.13	D	4.19	D
10	S. Higuera St	Tank Farm Road	Granada Dr	NB	D	3.52	D	3.55	D
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	D	3.35	C	3.46	C
	S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	3.46	C	3.43	C
12	S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	D	3.28	С	3.92	D
12	S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	D	3.94	D	3.88	D
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	3.72	D	3.74	D
13	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	3.39	C	3.49	C
14	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	3.57	D	3.59	 D
1-7	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	3.83	D	3.91	D
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	D	3.29	C	3.34	C
13	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	D	3.55	D	3.62	D
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	D	3.72	D	3.69	D
10	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	D	3.72	D	3.81	D
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	D	3.42	C	3.37	C
1,	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	D	3.42	С	3.40	С
1 2	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	D	3.14		3.40	
10	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	D	3.14	D	3.86	D
10	Froom Ranch Way	End	Los Osos Valley	WB	D	3.32	C	3.47	C
13	Froom Ranch Way		End	EB	D	2.87	С	3.49	С
20	Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	D	4.30	E	4.33	E
20	Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	D	4.30 3.79	D D	4.33	D D
	Notes:	TTOUTH NATION WAY	ividuUiiiid NU	IND	<u>и</u>	3./9	U	4.21	ט

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. No methodology exists for evaluating two-way stop-controlled interesction performance (with the cross-street stop controlled) for pedestrians and bicycles. However, it is incorporated into the methodology for evaluating bicycle segment performance.

TABLE 51:
NEAR TERM CONDITIONS SEGMENT LEVEL OF SERVICE: TRANSIT ANALYSIS

TRA	NSIT LOS						AM PI	AK	PM PE	AK
					LOS		SEGMENT		SEGMENT	
ID	Roadway	From	To	Direction	Threshold	Route Name	SCORE	LOS	SCORE	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	D	Ruote 4	4.15	D	4.28	E
_	Madonna Rd	LOVR	Oceanaire Dr	EB	D	Route 5	4.35	E	3.98	D
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	D	Route 4	4.47	E	4.62	E
	Madonna Rd	Oceanaire Dr	Dalidio	EB	D	Route 5	4.71	Ē	4.44	E
3	Madonna Rd	El Mercado	Dalidio Dr	WB	D	Route 4	4.29	Е	4.45	E
	Madonna Rd	Dalidio Dr	El Mercado	EB	D	0.00	Not Analyzed	N/A	Not Analyzed	N/A
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	Route 4	4.33	E	4.52	E
	Madonna Rd	El Mercado .	US 101 SB Ramps	EB	D	Route 5	4.58	E	4.32	E
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	Route 4	4.07	D	4.34	E
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	Route 5	4.22	D	3.78	D
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	Route 4	4.29	E	4.39	E
	Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	Route 5	4.49	E	4.13	D
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	D	Route 2	Not Analyzed	N/A	3.57	D
	S. Higuera St	Margarita Ave	Madonna Rd	NB	D	Route 2	3.72	, D	3.75	D
8	S. Higuera St	Margarita Ave	Prado Rd	SB	D	Route 2	Not Analyzed	N/A	4.21	D
_	S. Higuera St	Prado Rd	Margarita Ave	NB	D	Route 2	4.19	D	4.35	E
a	S. Higuera St	Prado Rd	Granada Dr	SB	D	Route 2	4.41	E	4.33	
,	S. Higuera St	Granada Dr	Prado Rd	NB	D	Route 2	3.77	D	3.96	D
10	S. Higuera St	Granada Dr	Tank Farm Road	SB	D	Route 2	3.82	D	3.91	
10	-		Granada Dr	NB	D	Route 2	3.54	D	3.64	D
11	S. Higuera St	Tank Farm Road				Route 2				
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
4.2	S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	Route 2	4.03	D	4.00	D
12	S. Higuera St	Suburban Drive	Los Osos Valley	SB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
12	S. Higuera St Los Osos Valley	Los Osos Valley Road	Froom Ranch Way	NB SB	D D	Route 4	Not Analyzed 4.51	N/A E	Not Analyzed 4.56	N/A E
13	•	Froom Ranch Way	Madonna Rd	NB	D	Route 4	4.25	D	4.43	E
13	Los Osos Valley	•	Froom Ranch Way	SB	D	Route 5	4.60	E	4.38	
13	•	Froom Ranch Way	Madonna Rd	NB	D	Route 5	4.40	E	4.21	D
14		Froom Ranch Way	Calle Joaquin	SB	D	Route 4	4.33	E	4.36	E
	Los Osos Valley	•	Froom Ranch Way	NB	D	Route 4	4.24	D	4.50	Ē
14	Los Osos Valley	•	Froom Ranch Way	SB	D	Route 5	4.44	E	4.12	 D
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 5	4.35	E	4.30	E
15	Los Osos Valley		US 101 SB Ramps	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	D		Not Analyzed	N/A	Not Analyzed	N/A
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	D		Not Analyzed	N/A	Not Analyzed	N/A
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Los Osos Valley		US 101 NB Ramps	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
18	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	D	Route 2	3.83	D	Not Analyzed	N/A
19	Froom Ranch	End	Los Osos Valley	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Froom Ranch	Los Osos Valley	End	EB	D		Not Analyzed	N/A	Not Analyzed	N/A
20	Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	D	Route 4	4.01	D	4.05	D
	Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	D	Route 5	4.47	E	4.35	E
	Notes:									

Segment 20 transit is southbound for routes 4 and 5

 $Route\ 2\,Serves\ the\ Prado\ Day\ Center\ stop\ during\ the\ AM\ peak\ hour,\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour$

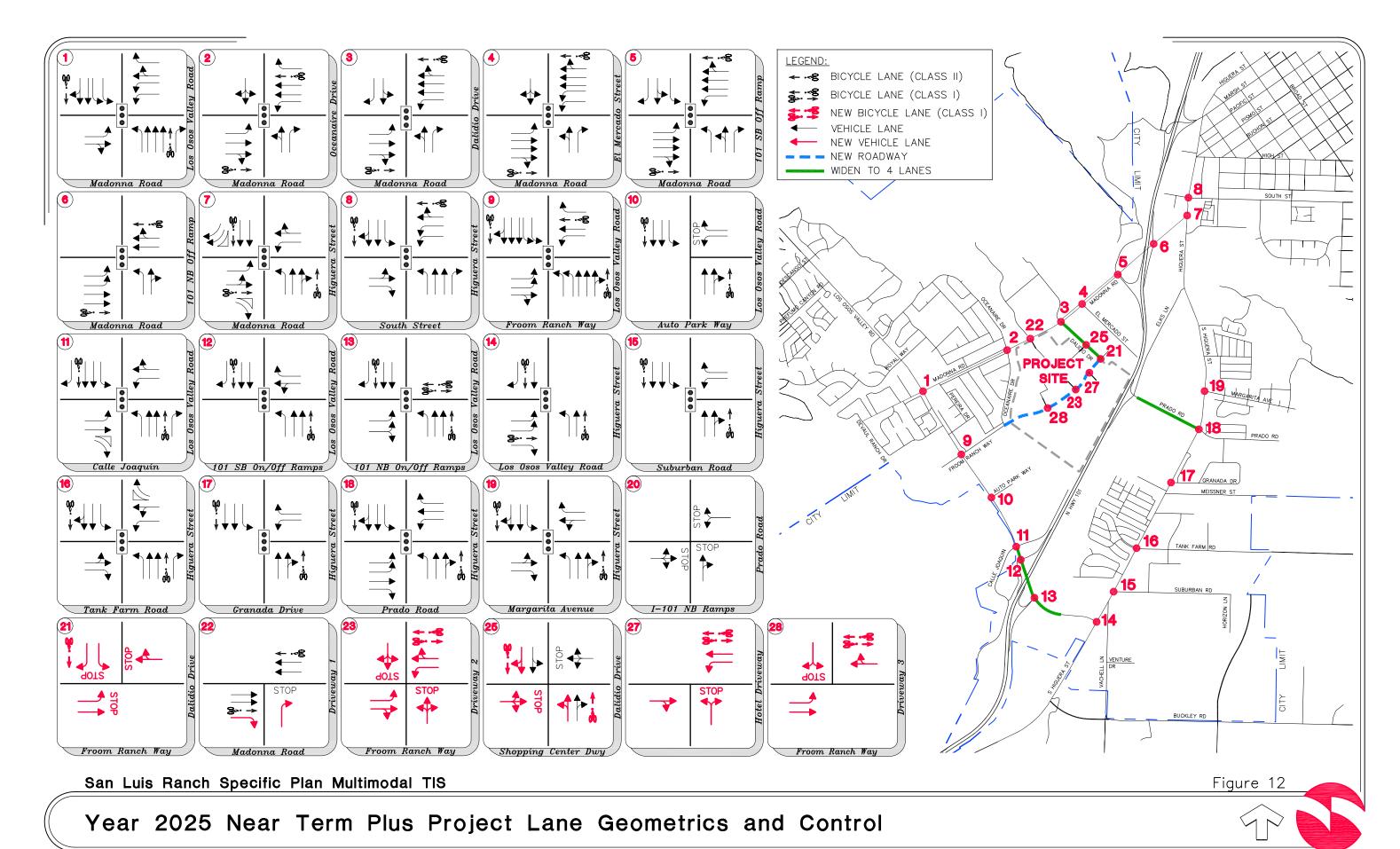
TABLE 52:
NEAR TERM CONDITIONS SEGMENT LEVEL OF SERVICE: FREEWAY ANALYSIS

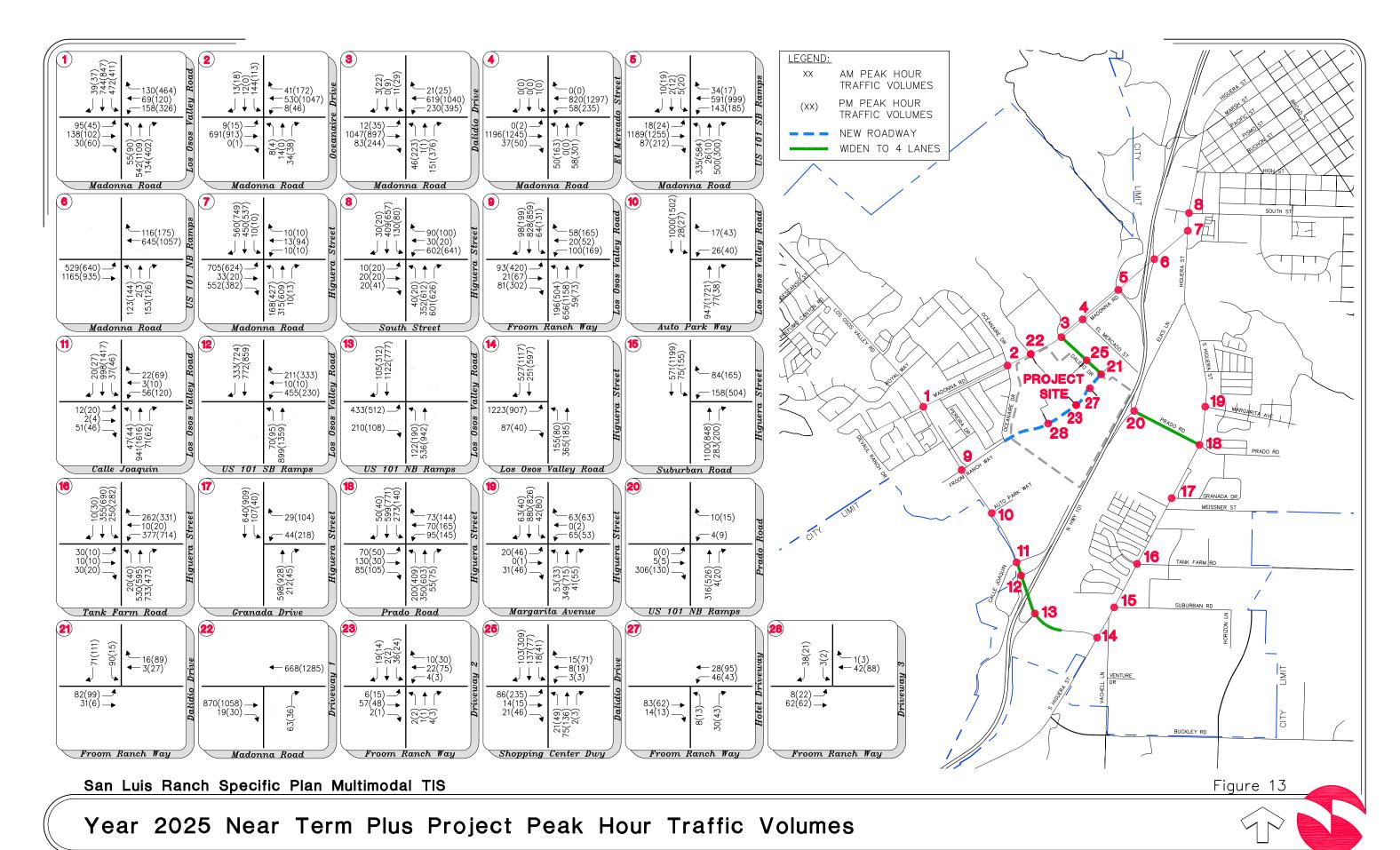
				AM Peak Hour			PM	Peak Hour	
	Target	Segment	No. of		Density			Density	
Interchange Location	LOS	Type	Lanes	Volume	(pc/mi/ln)	LOS	Volume	(pc/mi/ln)	LOS
US 101 at Los Osos Valley Road									
US 101 NB South of Los Osos Valley Road	С	Freeway	2	3,159	28.8	D	2,503	22.0	С
US 101 SB South of Los Osos Valley Road	С	Freeway	2	1,590	14.0	В	3,478	33.0	D
US 101 at Prado Road				•					
US 101 NB South of Prado Road	С	Freeway	2	2,750	24.3	С	2,393	21.0	С
US 101 at Madonna Road				•		-			
US 101 NB South of Madonna Road	С	Freeway	2	2,760	24.4	С	2,779	24.6	С
US 101 SB South of Madonna Road	С	Freeway	2	1,860	16.3	В	3,233	29.7	D

As shown in the Near Term Conditions segment analysis tables, most of the segments are currently operating at deficient LOS for pedestrian and transit modes, as well as several segments for automobile mode and only a couple segments for the bicycle mode. The segment of US 101 Northbound and Southbound south of Los Osos Valley Road and Southbound US 101 south of Madonna Road are operating at deficient LOS D during the Near Term conditions.

Near Term Plus Project Conditions

The project generated peak hour volumes have been added to the derived Near Term No Project volumes to obtain the Near Term Plus Project conditions. The mode split for the project added traffic remains the same as existing conditions. The LOS has been quantified and compared to all study intersections and roadways analyzed in Near Term No Project conditions. All proposed driveway intersections have been evaluated for LOS operations, potential vehicle queuing and pedestrian and bicycle level of service. Figure 12 presents the Near Term Plus Project lane geometrics and control assuming the proposed project is in place. Figure 13 presents the Near Term Plus Project peak hour traffic volumes with the proposed project in place.





Near Term Plus Project Conditions Analysis

The Near Term Plus Project conditions multimodal analysis for the study intersections and segments are presented below.

Near Term Plus Project Conditions Intersection Analysis

Table 53 provides a summary of the Near Term Plus Project conditions vehicular AM and PM peak hour intersection delay and LOS. Table 54 provides a summary of the Near Term Plus Project pedestrian AM and PM peak hour conditions at the study intersections. Table 55 provides a summary of the Near Term Plus Project bicycle AM and PM peak hour conditions at the study intersections. Table 56 provides a summary of the Near Term conditions queuing analysis.

TABLE 53:
NEAR TERM PLUS PROJECT CONDITIONS INTERSECTION LOS: AUTOMOBILE ANALYSIS

				AM Peak Hour			PN	l Peak H	our
#	Intersection	Control Type ^{1,2}	Target LOS	v/c³	Delay	LOS	v/c³	Delay	LOS
1	Madonna Road/Los Osos Valley Road	Signal	D		28.0	С	1.05	56.3	E
2	Madonna Road/Oceanaire Drive	Signal	D		21.3	С		19.0	В
3	Madonna Road/Dalidio Drive	Signal	D		47.0	D	2.82	156.5	F
4	Madonna Road/El Mercado	Signal	D		7.9	Α		20.0	В
5	Madonna Road/US 101 SB Ramps/Madonna	Signal	С	1.26	44.0	D		24.9	С
6	Madonna Road/US 101 NB Ramps	Signal	С		19.4	В		22.2	С
7	Madonna Road/Higuera Street	Signal	D		33.3	С		43.6	D
8	Higuera Street/South Street	Signal	D		27.7	С	1.43	81.0	F
9	Los Osos Valley Road/Froom Ranch Way	Signal	D		22.7	С		42.8	D
10	Los Osos Valley Road/Auto Park Way	TWSC	D		20.2	С	0.57	59.7	F
11	Los Osos Valley Road/Calle Joaquin	Signal	D		9.2	Α		6.6	Α
12	Los Osos Valley Road/US 101 SB Ramps	Signal	С		21.0	С		20.0	В
13	Los Osos Valley Road/US 101 NB Ramps	Signal	С		16.4	В		21.5	С
14	S. Higuera Street/Los Osos Valley Road	Signal	D		23.1	С		32.2	С
15	S. Higuera Street/Suburban Drive	Signal	D		8.6	Α		20.2	С
16	S. Higuera Street/Tank Farm Road	Signal	D	1.32	73.2	E		25.3	С
17	S. Higuera Street/Granada Drive	Signal	D		8.3	Α		11.5	В
18	S. Higuera Street/Prado Road	Signal	D		20.7	С		28.0	С
19	S. Higuera Street/Margarita Avenue	Signal	D		15.1	В		14.8	В
20	Prado Road/US 101 NB Ramps	AWSC	С		10.8	В		15.6	С
21	Froom Ranch Road/Dalidio Drive	AWSC	D		8.4	Α		8.4	Α
22	Madonna Road/Project Driveway	TWSC	D		12.7	В		13.6	В
23	Froom Ranch Road/Project Driveway#2	TWSC	D		9.3	Α		9.7	Α
25	Dalidio Drive/SC Project Driveway	TWSC	D		12.9	В	0.86	52.3	F
26	Dalidio Drive/Promenade	TWSC	D			Α			Α
27	Froom Ranch Road/Hotel Project Driveway	TWSC	D		9.2	Α		9.3	Α
28	Froom Ranch Road/Project Driveway#3	TWSC	D		8.7	Α		9.0	Α

Notes:

^{1.} AWSC = All Way Stop Control; TWSC = Two Way Stop Control; RNDBT = Roundabout

^{2.} LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC, Signal,

^{3.} Volume to Capacity Ratio (v/c) is for worst movement delay, for unacceptable LOS only

TABLE 54:
NEAR TERM PLUS PROJECT CONDITIONS INTERSECTION LOS: PEDESTRIAN ANALYSIS

				AM Peak	Hour	PM Peak Hour		
			Target	Ped. Crosswalk		Ped. Crosswalk		
#	Intersection	Approach	LOS	Score	LOS	Score	LOS	
		EB	С	2.11	В	2.13	В	
1	Madonna Road/Los Osos Valley Road	WB	С	2.90	С	3.32	С	
•	IMAUOIIIIA ROAU/LOS OSOS VAIIEY ROAU	NB	С	3.12	С	3.52	D	
		SB	С	3.21	C	3.31	C	
		EB WB	00	2.71	В	2.99	C C	
2	Madonna Road/Oceanaire Drive	NB	C	3.11 2.01	C B	3.39 2.01	В	
		SB	C	1.82	A	1.85	A	
		EB	C	3.03	Ĉ	3.44	Ĉ	
		WB	Ċ	3.05	Č	3.17	Č	
3	Madonna Road/Dalidio Drive	NB	С	2.46	В	2.60	В	
		SB	С	2.96	С	2.00	Α	
		EB	С	n/a	-	n/a	-	
4	Madonna Road/El Mercado	WB	С	3.11	С	3.19	С	
		NB	C	2.25	В	2.51	В	
		SB EB	C	1.71 3.08	A C	1.72 3.23	A C	
	Madonna Road/US 101 SB	WB	C	n/a	-	n/a	-	
5	Ramps/Madonna Inn	NB	Ċ	2.80	С	2.74	В	
	Transportation and	SB	С	2.18	В	2.19	В	
		EB	C	n/a	-	n/a	-	
G	Madanna Boad/IIS 101 ND Bonna	WB	С	2.96	С	2.92	С	
6	Madonna Road/US 101 NB Ramps	NB	С	2.02	В	2.02	В	
		SB	С	n/a	-	n/a	-	
		EB	С	2.96	С	3.22	С	
7	Madonna Road/Higuera Street	WB	С	2.01	В	2.02	В	
		NB CB	C	2.80	С	2.93	С	
		SB EB		n/a 2.03	- D	n/a	- D	
		WB EB	C	2.03	B C	2.06 2.83	B C	
8	Higuera Street/South Street	NB	C	n/a	-	n/a	-	
		SB	C	2.52	В	2.62	В	
		EB	C	2.51	В	2.80	C	
	Los Osos Valley Road/Froom Ranch	WB	Ċ	2.45	В	2.65	В	
9	Way	NB	С	n/a	-	n/a	-	
		SB	С	3.11	С	3.34	С	
		EB	С		-		-	
10	Los Osos Valley Road/Auto Park Way	WB	С	n/a	-	n/a	-	
	200 0000 valley Rodall allo Falk Way	NB	С	11/4	-	11/4	-	
		SB	С	0.47	-	0.00	-	
		EB WB	C	2.47 2.08	B B	2.28 2.18	B B	
11	Los Osos Valley Road/Calle Joaquin	NB	C	3.03	С	3.31	C	
		SB	C	3.03	C	3.29	C	
		EB	C	1.86	A	2.24	В	
	Los Osos Valley Road/US 101 SB	WB	Č	2.15	В	2.08	В	
12	Ramps	NB	С	n/a	-	n/a	-	
		SB	С	n/a	-	n/a	-	
_	Los Osos Valley Road/US 101 NB	EB	С	2.35	В	2.37	В	
13	Ramps	NB	С	2.78	С	2.79	С	
		SB	С	n/a	-	n/a	-	
		EB	С	3.00	С	2.92	С	
14	S. Higuera Street/Los Osos Valley Road	NB	С	2.38	В	2.41	В	
		SB	С	n/a	-	n/a	-	
15	S. Higuera Street/Suburban Drive	WB NB	С С	2.33 3.25	B C	2.57 3.98	В D	
	oguera ou een guburbatt brive	SB	C	2.88	C	3.90	C	
		EB	С	2.03	В	2.02	В	
16	S. Higuera Street/Tank Farm Road	WB	С	3.01	С	3.20	С	
	guota Guode talik i allii Noad	NB	С	3.46	С	3.39	С	
		SB	С	2.77	C	2.91	C	
17	S. Higuera Street/Granada Drive	WB	С	2.11	В	2.18	B -	
. /	O. I nguera Oueer Oraliada Dilve	NB SB	O O	n/a 2.71	- В	n/a 2.89	C	
		EB	C	2.66	В	2.72	В	
		WB	C	2.40	В	2.49	В	
18	S. Higuera Street/Prado Road	NB	C	2.90	C	3.19	C	
		SB	Ċ	2.86	Ċ	2.93	Č	
		EB	Č	2.28	В	2.09	В	
10	S Higuara Stroot/Margarita Avanua	WB	С	2.14	В	2.22	В	
19	S. Higuera Street/Margarita Avenue	NB	С	2.85	С	2.94	С	
_		SB	С	2.78	С	2.93	С	
_		EB	С	1	-		-	
20	Prado Road/US 101 NB Ramps	WB	С	n/a	-	n/a	-	
		NB SB	C		-		-	

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. HCM 2010 Methodologies for the pedestrian mode at two-way stop-controlled intersections is limited to the uncontrolled crossing. No methodology exists for evaluating pedestrian performance for the stop controlled approach (cross-street). However, it is reasoned that this type of control has negligible influence on pedestrian service along the segment.

TABLE 55: NEAR TERM PLUS PROJECT CONDITIONS INTERSECTION LOS: BICYCLE ANALYSIS

				AM Pea	k Hour	PM Peak Hour		
			Target	Bicycle LOS		Bicycle LOS		
ŧ	Intersection	Approach	LOS	Score	LOS	Score	LOS	
		EB WB	D	3.29 3.39	C C	3.18 4.30	C E	
1	Madonna Road/Los Osos Valley Road		D	1				
		NB	D	1.70	Α	2.18	В	
		SB	D	2.72	В	2.71	В	
		EB	D	2.74	В	2.96	С	
2	Madonna Road/Oceanaire Drive	WB	D	1.17	Α	1.79	Α	
_	Iwadonna reda, occanane brive	NB	D	2.76	С	2.74	В	
		SB	D	2.23	В	2.23	В	
		EB	D	2.23	В	2.13	В	
		WB	D	1.66	Α	1.94	Α	
3	Madonna Road/Dalidio Drive	NB	D	2.19	В	2.84	С	
		SB	D	1.77	A	1.84	A	
		EB	D	1.89	A	1.84	A	
		WB	D	1.85	A	2.16	В	
4	Madonna Road/El Mercado			1				
		NB	D	3.23	С	3.81	D	
		SB	D	3.03	С	3.03	С	
		EB	D	2.07	В	2.12	В	
5	Madonna Road/US 101 SB	WB	D	1.74	Α	1.96	Α	
,	Ramps/Madonna Inn	NB	D	n/a	-	n/a	-	
		SB	D	2.91	С	2.96	С	
		EB	D	2.78	C	2.45	В	
6	Madonna Road/US 101 NB Ramps	WB	D	1.79	A	2.09	В	
		NB	D	n/a	-	n/a	-	
		EB	D	3.56	 D	3.01	C	
						1		
7	Madonna Road/Higuera Street	WB	D	2.46	В	2.60	В	
	_	NB	D	1.81	Α	2.29	В	
		SB	D	2.37	В	2.59	В	
		EB	D	2.72	В	2.78	С	
_	11: 01 1/0 1/01 1	WB	D	2.83	С	2.94	С	
8	Higuera Street/South Street	NB	D	3.09	С	3.35	С	
		SB	D	1.62	A	1.80	A	
		EB	D	3.44	C	4.45	Ē	
	Las Osas Valley Boad/Freem Banch	WB	D	2.00	A	2.67	В	
9	Los Osos Valley Road/Froom Ranch							
	Way	NB	D	1.86	Α	2.31	В	
		SB	D	1.78	Α	1.87	Α	
		EB	D		-		-	
ın	Los Osos Valley Road/Auto Park Way	WB	D	n/a	-	n/a	-	
10	Los Osos valley (Cadinato i aik vvay	NB	D	11/4	-	11/4	-	
		SB	D		_		-	
		EB	D	2.99	С	2.99	С	
		WB	D	3.08	C	3.27	С	
11	Los Osos Valley Road/Calle Joaquin	NB	D	1.57	A	2.06	В	
		SB	D			0.84		
		WB	D	0.53	Α		Α	
_	Los Osos Valley Road/US 101 SB			n/a	-	n/a	-	
2	Ramps	NB	D	2.89	С	3.23	С	
	·	SB	D	1.69	Α	2.02	В	
	Los Osos Valley Road/US 101 NB	EB	D	n/a	-	n/a	-	
3	Ramps	NB	D	1.95	Α	2.37	В	
_		SB	D	2.88	С	2.76	С	
		EB	D	2.19	В	1.86	Α	
4	S. Higuera Street/Los Osos Valley Road	NB	D	2.20	В	1.74	Α	
	J	SB	D	2.63	В	4.24	D	
		WB	D	1.09	A	1.82	A	
=	S. Higuera Street/Suburban Drive		D			1		
J	o. Inguera oneerouburban Dirve	NB SB		2.43	В	2.10	В	
		SB	D	1.82	A	2.41	<u>B</u>	
		EB	D	2.72	В	2.67	В	
16	S. Higuera Street/Tank Farm Road	WB	D	2.65	В	3.33	С	
-	g.z.z zz zovia ami nodu	NB	D	2.39	В	2.21	В	
		SB	D	1.78	Α	2.10	В	
		WB	D	2.63	В	3.07	С	
7	S. Higuera Street/Granada Drive	NB	D	1.90	Α	2.05	В	
		SB	D	1.99	A	2.17	В	
-		EB	D				В	
				2.48	В	2.30		
8	S. Higuera Street/Prado Road	WB	D	2.93	С	3.31	С	
	-	NB	D	1.76	Α	2.19	В	
_		SB	D	2.05	В	2.07	В	
		EB	D	2.51	В	2.58	В	
	C. Historia Charletta	WB	D	2.74	В	2.72	В	
9	S. Higuera Street/Margarita Avenue	NB	D	1.68	A	2.00	A	
		SB	D		В	2.22	В	
_		EB	D	2.25	<u>B</u>	4.44	D	
					-		-	
0	Prado Road/US 101 NB Ramps	WB	D	n/a	-	n/a	-	
	·	NB	D		-		-	
	i	SB	D	1	-	1	-	

Notes:

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective.

No methodology exists for evaluating bicycle performance at two-way stop-controlled intersections. However, it is reasoned that this type of control has negligible influence on bicycle service along the segment for stop controll on the cross-street.

TABLE 56:
NEAR TERM PLUS PROJECT CONDITIONS 95TH PERCENTILE QUEUING ANALYSIS

Intersection				Total	95 th Percentile Queue/Lane (ft)		
ID	Location	Movement	No. Lanes	Storage (ft) ¹	AM Peak Hour	PM Peak Hour	
1	Madonna Road/Los	Northbound Right	1	175	100	261	
2	Madonna	Westbound Right	1	100	58	140	
3	Madonna Road/Dalidio	Westbound Left	1	275	194	336	
3	Drive	Westbound Through/Right	3	570	108	610	
4	Madonna Road/El	Westbound Left	2	260	50	535	
5	Madonna Road/US 101	Eastbound Left	1	100	83	111	
5	SB Ramps/Madonna Inn	Westbound Left	1	260	170	539	
6	Madonna Road/US 101	Northbound Left	1	185	156	258	
		Eastbound Right	1	150	248	314	
7	Madonna Road/Higuera	Northbound Left	1	160	185	376	
7	Street	Southbound Left/Through	2	250	159	430	
		Southbound Right	2	340	164	592	
		Westbound Left	2	240	342	1024	
	Higuera Street/South Street	Northbound Left	1	60	102	44	
8		Northbound Right	1	60	104	127	
		Southbound Left	1	70	116	122	
9	Los Osos Valley	Westbound Right	1	50	60	119	
	Los Osos Valley	Westbound Left/Through	1	180	259	216	
12	Road/US 101 SB	Southbound Through	1	240	293	273	
	Ramps	Southbound Right	1	125	188	177	
13	Los Osos Valley	Southbound Right	1	130	244	248	
14	S. Higuera Street/Los	Eastbound Right	1	90	189	156	
15	S. Higuera	Southbound Left	1	200	149	259	
40	S. Higuera Street/Tank	Northbound Right	1	100	186	170	
16	Farm Road	Southbound Left	1	165	211	243	
17	S. Higuera	Southbound Left	1	80	85	65	
	_	Westbound Left	1	105	110	131	
18	S. Higuera Street/Prado	Westbound Right	1	100	49	137	
	Road	Northbound Left	1	100	158	186	
40	S. Higuera	Northbound Left	1	60	78	70	
19	Street/Margarita Avenue	Southbound Left	1	60	83	99	
24	Prado Road/US 101 SB		1	250	158	424	

Notes: 1. Bolded entries indicate queues exceed available storage

- 2. Storage Length of " " represents a lane which exceeds 900 feet, usually a through lane.
- 3. For Movements with more than one lane, the maximum of the 95th percentile queue is reported.

As shown in Table 53, the intersections of Los Osos Valley Road/Madonna Road, Madonna Road/Dalidio Drive, Madonna Road/US 101 SB Ramps/Madonna Inn, Higuera Street/South Street, Los Osos Valley Road/Auto Park Way, and S. Higuera Street/Tank Farm Road are projected to operate at unacceptable conditions during Near Term Plus Project Conditions. . Queuing analysis results show numerous locations in which the 95th percentile queue are expected to exceed the available storage capacity during Near Term Plus Project conditions.

Pedestrian analysis shows acceptable conditions at the study intersections. Bicycle analysis shows deficiencies at of Los Osos Valley Road/Madonna Road and at Los Osos Valley

^{4. *} Represents storage lengths for one lane; second lane is a left or right trap lane.

Road/Froom Ranch Way. Transportation improvements required to mitigate project-related impacts are detailed in a subsequent section of this report.

Near Term Plus Project Conditions Segment Analysis

Table 57 provides a summary of the Near Term Plus Project vehicular AM and PM peak hour conditions for the study segments. Table 58 provides a summary of the Near Term Existing Plus Project pedestrian AM and PM peak hour conditions for the study segments. Table 59 provides a summary of the Near Term Plus Project bicycle AM and PM peak hour conditions for the study segments. Table 60 provides a summary of the Near Term Plus Project transit AM and PM peak hour conditions for the study segments. Table 61 provides a summary of the Near Term Plus Project freeway segments analysis for AM and PM peak hour conditions for the study segments along US 101. Transportation improvements required to mitigate project-related impacts are detailed in a subsequent section of this report.

TABLE 57:
NEAR TERM PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: AUTOMOBILE ANALYSIS

UTO SE	EGMENT LOS						AM I	PEAK		PM PEAK				
					LOS		Base Free-			Travel	Base Free-	Travel		
					Threshold	Travel Speed	Flow Speed	Travel Speed/		Speed	Flow Speed	Speed/		
) Road	dway	From	То	Direction	Tillesilolu	(mph)	BFFS (mph)	BFFS (%)	LOS	(mph)	BFFS (mph)	BFFS (%)	LOS	
Mad	donna Rd	Oceanaire Dr	LOVR	WB	С	21.7	42.1	51%	С	10.2	42.1	24%	F	
Mad	donna Rd	LOVR	Oceanaire Dr	EB	С	28.0	42.1	67%	С	28.5	42.1	68%	В	
Mad	donna Rd	Dalidio	Oceanaire Dr	WB	С	23.6	40.8	58%	С	23.8	40.7	59%	С	
Mad	donna Rd	Oceanaire Dr	Dalidio	EB	С	11.5	40.7	28%	F	16.3	40.8	40%	E	
Mad	donna Rd	El Mercado	Dalidio Dr	WB	С	14.9	34.1	44%	D	14.2	34.8	41%	D	
Mad	donna Rd	Dalidio Dr	El Mercado	EB	С	26.0	38.2	68%	В	11.5	34.6	33%	E	
Mad	donna Rd	US 101 SB Ramps	El Mercado	WB	С	30.8	37.9	81%	В	22.0	37.3	59%	С	
Mad	donna Rd	El Mercado	US 101 SB Ramps	EB	С	20.7	37.8	55%	С	17.5	37.7	46%	D	
Mad	donna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	С	28.4	37.8	75%	В	22.3	37.8	59%	С	
Mad	donna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	С	32.8	37.8	87%	Α	33.3	37.8	88%	Α	
Mad	donna Rd	Higuera St	US 101 NB Ramps	WB	С	10.6	37.2	29%	F	14.1	37.2	38%	E	
Mad	donna Rd	US 101 NB Ramps	Higuera St	EB	С	12.6	37.2	34%	E	8.0	37.2	21%	F	
_	liguera St	Madonna Rd	Margarita Ave	SB	C	32.6	44.5	73%	В	33.1	44.5	74%	В	
	liguera St	Margarita Ave	Madonna Rd	NB	С	33.0	44.8	74%	В	33.5	44.8	75%	В	
	liguera St	Margarita Ave	Prado Rd	SB	c	16.2	38.9	42%		11.5	38.9	30%	F	
	liguera St	Prado Rd	Margarita Ave	NB	c	21.2	38.9	55%	c	19.1	38.9	49%	D	
	liguera St	Prado Rd	Granada Dr	SB	С	33.5	41.8	80%	В	29.3	41.8	70%	В	
	iguera St	Granada Dr	Prado Rd	NB	c	21.6	41.9	52%	C	23.9	41.9	57%	C	
	iguera St	Granada Dr	Tank Farm Road	SB	С	42.3	41.6	102%	A	24.7	42.6	58%	C	
	iguera St	Tank Farm Road	Granada Dr	NB	c	29.7	41.6	71%	В	26.8	42.6	63%	C	
_	iguera St	Tank Farm Road	Suburban Drive	SB	С	27.1	42.4	64%	C	21.5	41.2	52%	C	
	-				c	17.8			D	16.2			E	
	liguera St	Suburban Drive	Tank Farm Road	NB CD			42.5	42% 49%	D		41.3	39% 34%	<u>-</u>	
	liguera St	Suburban Drive	Los Osos Valley Road	SB	С	20.4	42.1			13.4	39.1		_	
_	liguera St	Los Osos Valley Road		NB	С	23.7	42.0	56%	C	18.9	39.0	49%	D	
	Osos Valley	Madonna Rd	Froom Ranch Way	SB	С	22.7	41.9	54%	C	15.2	41.8	36%	E	
	Osos Valley	Froom Ranch Way	Madonna Rd	NB	С	20.5	41.8	49%	D	14.8	41.8	36%	E	
	Osos Valley	Froom Ranch Way	Calle Joaquin	SB	С	29.0	43.0	67%	В	30.3	43.0	71%	В	
	Osos Valley	Calle Joaquin	Froom Ranch Way	NB	С	28.1	43.2	65%	С	21.4	43.2	50%	D	
	Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	С	6.2	32.1	19%	F	13.1	32.1	41%	D	
	Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	С	16.2	31.1	52%	С	13.9	31.1	45%	D	
Los	Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	С	24.7	37.7	66%	С	21.0	37.7	56%	С	
Los	Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	С	20.2	37.4	54%	С	32.2	37.4	86%	Α	
Los (Osos Valley	US 101 NB Ramps	S. Higuera St	EB	С	16.9	39.4	43%	D	15.6	39.4	39%	E	
Los	Osos Valley	S. Higuera St	US 101 NB Ramps	WB	С	24.2	39.2	62%	С	26.7	39.2	68%	В	
Prac	do Rd	S. Higuera St	US 101 NB Ramps	WB	С	27.4	39.1	70%	В	23.9	39.1	61%	С	
Prad	do Rd	US 101 NB Ramps	S. Higuera St	EB	С	23.4	39.0	60%	С	21.7	39.0	56%	С	
Froc	om Ranch Way	Dick's Sporting Goods	s Los Osos Valley	WB	С	17.8	38.0	47%	D	13.0	37.4	35%	E	
Froc	om Ranch Way	Los Osos Valley	Dick's Sporting Goods [EB	С	34.7	37.4	93%	Α	34.9	37.9	92%	Α	
	dio Dr	Madonna Rd	Froom Ranch Way	SB	С	29.3	39.3	74%	В	29.5	39.3	75%	В	
0 Dalid					1 _	44.3	27.0	30%	F		27.0	30/	F	
Dalid	idio Dr	Froom Ranch Way	Madonna Rd	NB	С	11.2	37.8	30%	r	1.1	37.8	3%		
0 Dalid	idio Dr om Ranch Way	Proom Ranch Way Dalidio	Madonna Rd Dick's Sporting Goods [NB WB	D	40.0	40.6	98%	A A	39.9	40.6	98%	A	

TABLE 58:
NEAR TERM PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: PEDESTRIAN ANALYSIS

PEC	ESTRIAN SEGMENT	LOS					AM PI	EAK	PM PI	EAK
					LOS	Average Ped.	SEGMENT		SEGMENT	
ID	Roadway	From	То	Direction	Threshold	Space (ft ² /p)	SCORE	LOS	SCORE	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	С	2648	3.54	D	3.88	D
	Madonna Rd	LOVR	Oceanaire Dr	EB	С	17482	3.93	D	4.10	D
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	С	12000	3.64	D	4.00	D
	Madonna Rd	Oceanaire Dr	Dalidio	EB	С	5833	3.84	D	3.97	D
3	Madonna Rd	El Mercado	Dalidio Dr	WB	С	37450	3.61	D	3.99	D
	Madonna Rd	Dalidio Dr	El Mercado	EB	С	52920	3.74	D	3.89	D
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	С	26250	3.68	D	3.87	D
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	С	27915	3.77	D	3.87	D
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	С	No Peds	3.76	D	3.95	F
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	c	No Peds	4.12	D	4.07	D
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	C	25200	3.70		3.88	D
٠	Madonna Rd	US 101 NB Ramps	Higuera St	EB	c	19838	3.90	D	3.86	D
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	C	23247	3.87	D	3.90	D
•	S. Higuera St	Margarita Ave	Madonna Rd	NB	c	5398	3.68	D	3.92	D
8	S. Higuera St	Margarita Ave	Prado Rd	SB	С	40979	3.71	D	3.74	D
Ŭ	S. Higuera St	Prado Rd	Margarita Ave	NB	c	21700	3.55	D	3.71	D
۵	S. Higuera St	Prado Rd	Granada Dr	SB	C	9292	3.65	D	3.77	D
,	S. Higuera St	Granada Dr	Prado Rd	NB	c	8400	3.26	C	3.53	D
10		Granada Dr	Tank Farm Road	SB	c	46305	3.62	D	3.80	D
ΙU	S. Higuera St		Granada Dr	NB	C	46305 49140	3.62	C	3.36	C
	S. Higuera St	Tank Farm Road			c			D		D D
11	S. Higuera St	Tank Farm Road	Suburban Drive	SB	_	12600	3.66		3.96	_
	S. Higuera St	Suburban Drive Suburban Drive	Tank Farm Road	NB	С	31500	3.66	D D	3.56	D D
12	S. Higuera St		Los Osos Valley Road	SB NB	C C	39312	3.64 4.00	D	4.02 4.06	D
1 2	S. Higuera St	Los Osos Valley Road Madonna Rd		SB	С	43533	3.86	<u> </u>	3.99	<u>D</u>
13	Los Osos Valley		Froom Ranch Way			3853				_
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	С	0	3.75	<u> </u>	4.19	F
14	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	С	27300	3.87	D	4.08	D
	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	С	22050	3.75	D	4.11	D
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	С	44100	3.71	D	3.72	D
	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	С	63000	3.68	D	4.03	D
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	С	No Peds	3.94	D	3.92	D
	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	С	53928	3.83	D	3.29	С
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	С	46575	3.95	D	3.79	D
	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	С	1680	3.89	D	4.29	E
18	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	С	25200	2.76	С	2.85	С
	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	С	3019	3.44	С	3.42	С
19	Froom Ranch Way	Dick's Sporting Goods	Los Osos Valley	WB	С	4846	3.06	С	3.26	С
	Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods [EB	С	No Peds	1.79	Α	1.75	Α
20	Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	С	3500	1.64	Α	1.60	Α
	Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	С	21000	3.10	С	3.30	С
21	Froom Ranch Way	Dalidio	Dick's Sporting Goods [WB	С	8400	1.55	А	1.57	Α
	Froom Ranch Way	Dick's Sporting Goods		EB	С	No Peds	1.64	Α	1.64	Α

Sidewalk is present along frontage roads for segments #1 - Madonna Road and #13 - Los Osos Valley Road, and is not accounted for in this analysis.

HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. No methodology exists for evaluating two-way stop-controlled intersection performance (with the cross-street stop controlled) for pedestrians and bicycles. However, it is reasoned that it has negligible influence on pedestrian sevice along the segment.

TABLE 59:
NEAR TERM PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: BICYCLE ANALYSIS

D Roadway	BIC	YCLE SEGMENT LOS		AM PEA	AK	PM PEAK				
Madonna Rd										
Madonna Rd LOVR Oceanaire Dr EB D 3.73 D 3.80 D 2 Madonna Rd Dalidio Oceanaire Dr WB D 3.15 C 3.24 C 3 Madonna Rd El Mercado Dalidio Dr WB D 3.31 C 3.24 C 4 Madonna Rd Dalidio Dr El Mercado EB D 3.40 C 3.42 C 4 Madonna Rd US 101 SB Ramps El Mercado WB D 3.98 D 4.38 E 5 Madonna Rd LIS 101 NB Ramps US 101 SB Ramps US 101 SB Ramps WB D 3.64 D 3.65 D 6 Madonna Rd US 101 NB Ramps US 101 NB Ramps WB D 3.33 C 3.38 C 6 Madonna Rd Higuera St US 101 NB Ramps WB D 3.52 D 3.58 D 7 S. Higuera St Madonna Rd Margarita Ave SB D 3.91 D <td>ID</td> <td>•</td> <td>-</td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td>SCORE</td> <td></td>	ID	•	-	-					SCORE	
Madonna Rd	1	Madonna Rd	Oceanaire Dr	LOVR	WB	D	3.61	D	4.17	D
Madonna Rd Oceanaire Dr Dalidio Dr EB D 3.57 D 3.44 C 3 Madonna Rd El Mercado Dalidio Dr WB D 3.31 C 3.24 C 4 Madonna Rd US 101 SB Ramps El Mercado US D 3.98 D 4.38 E Madonna Rd El Mercado US 101 SB Ramps EB D 3.64 D 3.65 D 5 Madonna Rd HI OS 101 NB Ramps US 101 SB Ramps WB D 3.33 C 3.38 C 6 Madonna Rd Higuera St US 101 NB Ramps EB D 3.40 C 3.35 C 7 S. Higuera St Madonna Rd Higuera St US 101 NB Ramps Higuera St EB D 3.57 D 3.58 D 7 S. Higuera St Madonna Rd Margarita Ave Madonna Rd Margarita Ave SB D 3.91 D 3.90 D 4.19 D 3.90 D <		Madonna Rd	LOVR	Oceanaire Dr	EB	D	3.73	D	3.80	D
Madonna Rd El Mercado Dalidio Dr El Mercado EB D 3.40 C 3.24 C	2	Madonna Rd	Dalidio	Oceanaire Dr	WB	D	3.15	С	3.24	С
Madonna Rd		Madonna Rd	Oceanaire Dr	Dalidio	EB	D	3.57	D	3.44	С
Madonna Rd	3	Madonna Rd	El Mercado	Dalidio Dr	WB	D	3.31	С	3.24	С
Madonna Rd		Madonna Rd	Dalidio Dr	El Mercado	EB	D	3.40	С	3.42	С
Madonna Rd	4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	3.98	D	4.38	E
Madonna Rd US 101 SB Ramps US 101 NB Ramps EB D 3.40 C 3.35 C 6 Madonna Rd Higuera St US 101 NB Ramps WB D 3.52 D 3.58 D 7 S. Higuera St Madonna Rd Margarita Ave SB D 3.91 D 3.90 D 8 S. Higuera St Margarita Ave Madonna Rd NB D 4.09 D 4.19 D 8 S. Higuera St Margarita Ave Prado Rd SB D 3.67 D 3.67 D 9 S. Higuera St Prado Rd Margarita Ave NB D 3.67 D 3.67 D 9 S. Higuera St Granada Dr Frado Rd NB D 3.88 D 3.91 D 10 S. Higuera St Granada Dr Tank Farm Road SB D 3.45 C 3.53 D 11 S. Higuera St Tank Farm Road Suburban Drive SB D 3.35 C		Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	3.64	D	3.65	D
6 Madonna Rd Madonna Rd Malguera St Madonna Rd US 101 NB Ramps US 101 NB Ramps Higuera St EB D D 3.52 D D 3.58 D D 7 S. Higuera St Madonna Rd S. Higuera St Margarita Ave Madonna Rd S. Higuera St Margarita Ave Madonna Rd NB D 4.09 D 4.19 D D 3.90 D D 4.19 D D 3.67 D	5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	3.33	С	3.38	С
Madonna Rd US 101 NB Ramps Higuera St EB D 3.77 D 3.59 D 7 S. Higuera St Madonna Rd Margarita Ave SB D 3.91 D 3.90 D 8 S. Higuera St Margarita Ave Madonna Rd NB D 4.09 D 4.19 D 8 S. Higuera St Margarita Ave Prado Rd SB D 3.67 D 3.67 D 9 S. Higuera St Prado Rd Margarita Ave NB D 3.87 D 3.97 D 9 S. Higuera St Granada Dr Prado Rd NB D 3.45 C 3.53 D 10 S. Higuera St Granada Dr Trank Farm Road SB D 4.13 D 4.19 D 11 S. Higuera St Tank Farm Road Suburban Drive SB D 3.52 D 3.55 D 12 S. Higuera St Suburban Drive SB D 3.26 C 3.46 C<		Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	3.40	С	3.35	С
7 S. Higuera St Madonna Rd Margarita Ave SB D 3.91 D 3.90 D 8 S. Higuera St Margarita Ave Madonna Rd NB D 4.09 D 4.19 D 8 S. Higuera St Margarita Ave Prado Rd SB D 3.67 D 3.67 D 9 S. Higuera St Prado Rd Margarita Ave NB D 3.87 D 3.97 D 9 S. Higuera St Prado Rd Granada Dr SB D 3.88 D 3.91 D 10 S. Higuera St Granada Dr Tank Farm Road SB D 4.13 D 4.19 D 11 S. Higuera St Tank Farm Road Suburban Drive SB D 3.35 C 3.46 C 12 S. Higuera St Suburban Drive Tank Farm Road NB D 3.28 C 3.43 C 12 S. Hig	6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	3.52	D	3.58	D
S. Higuera St Margarita Ave Madonna Rd NB D 4.09 D 4.19 D 8 S. Higuera St Margarita Ave Prado Rd SB D 3.67 D 3.67 D 9 S. Higuera St Prado Rd Margarita Ave NB D 3.87 D 3.97 D 9 S. Higuera St Prado Rd Granada Dr SB D 3.88 D 3.91 D 10 S. Higuera St Granada Dr Tank Farm Road SB D 4.13 D 4.19 D 1. S. Higuera St Tank Farm Road Granada Dr NB D 3.52 D 3.55 D 1. S. Higuera St Tank Farm Road Suburban Drive SB D 3.35 C 3.46 C 3.43 C 2. Higuera St Suburban Drive Tank Farm Road NB D 3.28 C 3.94 D 3. Higuera St Suburban Drive Los Osos Valley Road SB <td></td> <td>Madonna Rd</td> <td>US 101 NB Ramps</td> <td>Higuera St</td> <td>EB</td> <td>D</td> <td>3.77</td> <td>D</td> <td>3.59</td> <td>D</td>		Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	3.77	D	3.59	D
S. Higuera St Margarita Ave Madonna Rd NB D 4.09 D 4.19 D 8 S. Higuera St Margarita Ave Prado Rd SB D 3.67 D 3.67 D 9 S. Higuera St Prado Rd Margarita Ave NB D 3.87 D 3.97 D 9 S. Higuera St Prado Rd Granada Dr SB D 3.88 D 3.91 D 10 S. Higuera St Granada Dr Tank Farm Road SB D 4.13 D 4.19 D 1. S. Higuera St Tank Farm Road Granada Dr NB D 3.52 D 3.55 D 1. S. Higuera St Tank Farm Road Suburban Drive SB D 3.35 C 3.46 C 3.43 C 2. Higuera St Suburban Drive Tank Farm Road NB D 3.28 C 3.94 D 3. Higuera St Suburban Drive Los Osos Valley Road SB <td>7</td> <td>S. Higuera St</td> <td>Madonna Rd</td> <td>Margarita Ave</td> <td>SB</td> <td>D</td> <td>3.91</td> <td>D</td> <td>3.90</td> <td>D</td>	7	S. Higuera St	Madonna Rd	Margarita Ave	SB	D	3.91	D	3.90	D
8 S. Higuera St Margarita Ave Prado Rd SB D 3.67 D 3.67 D 5. Higuera St Prado Rd Margarita Ave NB D 3.87 D 3.97 D 9 S. Higuera St Prado Rd Granada Dr SB D 3.88 D 3.91 D 10 S. Higuera St Granada Dr Trank Farm Road SB D 4.13 D 4.19 D 5. Higuera St Trank Farm Road Granada Dr NB D 3.52 D 3.55 D 11 S. Higuera St Trank Farm Road Suburban Drive SB D 3.35 C 3.46 C 5. Higuera St Suburban Drive Trank Farm Road NB D 3.46 C 3.43 C 12 S. Higuera St Suburban Drive NB D 3.28 C 3.43 C 15 Liguera St Los Osos Valley Madonna Rd			Margarita Ave		NB	D	4.09	D	4.19	D
S. Higuera St Prado Rd Margarita Ave NB D 3.87 D 3.97 D 9 S. Higuera St Prado Rd Granada Dr SB D 3.88 D 3.91 D 10 S. Higuera St Granada Dr Tank Farm Road SB D 4.13 D 4.19 D 1. Higuera St Tank Farm Road Granada Dr NB D 3.52 D 3.55 D 1. Higuera St Tank Farm Road Suburban Drive SB D 3.35 C 3.46 C S. Higuera St Suburban Drive Tank Farm Road NB D 3.46 C 3.43 C 1. S. Higuera St Suburban Drive Tank Farm Road NB D 3.28 C 3.46 C 3.43 C 1. S. Higuera St Suburban Drive NB D 3.28 C 3.94 D 3.88 D 3.28 C 3.94 D 3.88 D 3.2	8		-	Prado Rd	SB	D	3.67	D	3.67	
9 S. Higuera St S. Higuera St Suburban Drive S. Higuera St Suburban Drive S. Higuera St Suburban Drive S. Higuera St Suburban Drive Suburban Dr			-	Margarita Ave	NB	D	3.87	D	3.97	D
S. Higuera St Granada Dr Prado Rd NB D 3.45 C 3.53 D 10 S. Higuera St Granada Dr Tank Farm Road SB D 4.13 D 4.19 D S. Higuera St Tank Farm Road Granada Dr NB D 3.52 D 3.55 D 11 S. Higuera St Tank Farm Road Suburban Drive SB D 3.35 C 3.46 C S. Higuera St Suburban Drive Tank Farm Road NB D 3.46 C 3.43 C 12 S. Higuera St Suburban Drive Los Osos Valley Road SB D 3.28 C 3.94 D S. Higuera St Los Osos Valley Road Suburban Drive NB D 3.28 C 3.94 D S. Higuera St Los Osos Valley Road Suburban Drive NB D 3.72 D 3.74 D 13 Los Osos Valley Madonna Rd Fr	9		Prado Rd	-		D		D		
S. Higuera St Granada Dr Tank Farm Road SB D 4.13 D 4.19 D		· ·				D		С		D
S. Higuera St Tank Farm Road Granada Dr NB D 3.52 D 3.55 D 11 S. Higuera St Tank Farm Road Suburban Drive SB D 3.35 C 3.46 C S. Higuera St Suburban Drive Los Osos Valley Road SB D 3.28 C 3.94 D S. Higuera St Los Osos Valley Road Suburban Drive NB D 3.28 C 3.94 D S. Higuera St Los Osos Valley Road Suburban Drive NB D 3.94 D 3.88 D S. Higuera St Los Osos Valley Madonna Rd Froom Ranch Way SB D 3.72 D 3.74 D Los Osos Valley Froom Ranch Way Madonna Rd NB D 3.39 C 3.49 C 14 Los Osos Valley Froom Ranch Way Madonna Rd NB D 3.57 D 3.60 D Los Osos Valley Calle Joaquin Froom Ranch Wa	10									
S. Higuera St Tank Farm Road Suburban Drive SB D 3.35 C 3.46 C S. Higuera St Suburban Drive Tank Farm Road NB D 3.46 C 3.43 C C S. Higuera St Suburban Drive Los Osos Valley Road SB D 3.28 C 3.94 D S. Higuera St Los Osos Valley Road Suburban Drive NB D 3.94 D 3.88 D D 3.94 D 3.89 D 3.74 D D 3.94 D 3.89 D 3.74 D D 3.94 D 3.74 D D 3.75 D 3.60 D D D D D D D D D		-	Tank Farm Road	Granada Dr	_					
S. Higuera St Suburban Drive Tank Farm Road NB D 3.46 C 3.43 C 12 S. Higuera St Suburban Drive Los Osos Valley Road SB D 3.28 C 3.94 D S. Higuera St Los Osos Valley Road Suburban Drive NB D 3.94 D 3.88 D 13 Los Osos Valley Madonna Rd Froom Ranch Way SB D 3.72 D 3.74 D Los Osos Valley Froom Ranch Way Madonna Rd NB D 3.39 C 3.49 C 14 Los Osos Valley Froom Ranch Way Madonna Rd NB D 3.57 D 3.60 D 14 Los Osos Valley Calle Joaquin Froom Ranch Way NB D 3.57 D 3.60 D 15 Los Osos Valley Calle Joaquin US 101 SB Ramps SB D 3.29 C 3.35 C Los Osos Valley <	11									
12 S. Higuera St Suburban Drive Los Osos Valley Road SB D 3.28 C 3.94 D 13 Los Osos Valley Madonna Rd Froom Ranch Way SB D 3.72 D 3.74 D 14 Los Osos Valley Froom Ranch Way Madonna Rd NB D 3.39 C 3.49 C 14 Los Osos Valley Froom Ranch Way Madonna Rd NB D 3.39 C 3.49 C 14 Los Osos Valley Froom Ranch Way Madonna Rd NB D 3.39 C 3.49 C 15 Los Osos Valley Calle Joaquin Froom Ranch Way NB D 3.57 D 3.60 D 15 Los Osos Valley Calle Joaquin US 101 SB Ramps SB D 3.29 C 3.35 C 16 Los Osos Valley US 101 SB Ramps Calle Joaquin NB D 3.72 D 3.62 D 16 Los Osos Valley US 101 SB Ramps US 101 NB Ramps SB		· ·			_					
S. Higuera St Los Osos Valley Road Suburban Drive NB D 3.94 D 3.88 D 13 Los Osos Valley Madonna Rd Froom Ranch Way SB D 3.72 D 3.74 D Los Osos Valley Froom Ranch Way Madonna Rd NB D 3.39 C 3.49 C 14 Los Osos Valley Froom Ranch Way Calle Joaquin SB D 3.57 D 3.60 D Los Osos Valley Calle Joaquin Froom Ranch Way NB D 3.83 D 3.91 D Los Osos Valley Calle Joaquin US 101 SB Ramps SB D 3.29 C 3.35 C Los Osos Valley US 101 SB Ramps Calle Joaquin NB D 3.55 D 3.62 D 16 Los Osos Valley US 101 SB Ramps US 101 NB Ramps SB D 3.72 D 3.69 D Los Osos Valley US 101 NB Ramps S	12									
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HCM 2010 Methodologies do not model segments bounded by all-way stop control. Procedures have not been developed yet to address the effect of all-way stop control or yield control on intersection performance from a pedestrian or bicyce perspective. No methodology exists for evaluating two-way stop-controlled interesction performance (with the cross-street stop controlled) for pedestrians and bicycles. However, it is incorporated into the methodology for evaluating bicycle segment performance.

TABLE 60: NEAR TERM PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: TRANSIT ANALYSIS

N	NSIT LOS						AM PE	AK	PM PE	AK
					LOS	Route Name	SEGMENT		SEGMENT	
D	Roadway	From	То	Direction	Threshold	Route Name	SCORE	LOS	SCORE	LOS
1	Madonna Rd	Oceanaire Dr	LOVR	WB	D	Ruote 4	4.16	D	4.28	E
	Madonna Rd	LOVR	Oceanaire Dr	EB	D	Route 5	4.34	E	4.01	D
2	Madonna Rd	Dalidio	Oceanaire Dr	WB	D	Route 4	4.47	Е	4.63	E
	Madonna Rd	Oceanaire Dr	Dalidio	EB	D	Route 5	4.70	E	4.44	E
3	Madonna Rd	El Mercado	Dalidio Dr	WB	D	Route 4	4.34	Е	4.49	Е
	Madonna Rd	Dalidio Dr	El Mercado	EB	D	Route 5	Not Analyzed	N/A	Not Analyzed	N/A
4	Madonna Rd	US 101 SB Ramps	El Mercado	WB	D	Route 4	4.35	Е	4.55	Е
	Madonna Rd	El Mercado	US 101 SB Ramps	EB	D	Route 5	4.60	E	4.35	E
5	Madonna Rd	US 101 NB Ramps	US 101 SB Ramps	WB	D	Route 4	4.09	D	4.38	E
	Madonna Rd	US 101 SB Ramps	US 101 NB Ramps	EB	D	Route 5	4.25	D	3.83	D
6	Madonna Rd	Higuera St	US 101 NB Ramps	WB	D	Route 4	4.31	Е	4.43	Е
	Madonna Rd	US 101 NB Ramps	Higuera St	EB	D	Route 5	4.51	E	4.16	D
7	S. Higuera St	Madonna Rd	Margarita Ave	SB	D	Route 2	Not Analyzed	N/A	3.58	D
•	S. Higuera St	Margarita Ave	Madonna Rd	NB	D	Route 2	3.72	D	3.75	D
0	S. Higuera St		Prado Rd	SB	D	Route 2	Not Analyzed	N/A	4.21	D
ō	· ·	Margarita Ave			_		· '	•		
_	S. Higuera St	Prado Rd	Margarita Ave	NB	D	Route 2	4.22	D	4.34	E
9	S. Higuera St	Prado Rd	Granada Dr	SB	D	Route 2	4.41	E	4.33	Е
	S. Higuera St	Granada Dr	Prado Rd	NB	D	Route 2	3.77	D	3.95	D
LO	S. Higuera St	Granada Dr	Tank Farm Road	SB	D	Route 2	3.82	D	3.91	D
	S. Higuera St	Tank Farm Road	Granada Dr	NB	D	Route 2	3.54	D	3.64	D
1	S. Higuera St	Tank Farm Road	Suburban Drive	SB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
	S. Higuera St	Suburban Drive	Tank Farm Road	NB	D	Route 2	4.03	D	4.00	D
12	S. Higuera St	Suburban Drive	Los Osos Valley Road	SB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
	S. Higuera St	Los Osos Valley Road	Suburban Drive	NB	D	-	Not Analyzed	N/A	Not Analyzed	N/A
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	Route 4	4.53	E	4.56	E
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 4	4.25	E	4.43	E
13	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	Route 5	4.61	Е	4.38	Е
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 5	4.36	E	4.21	D
4	Los Osos Valley	Froom Ranch Way	Calle Joaquin	SB	D	Route 4	4.34	E	4.37	Е
	Los Osos Valley	Calle Joaquin	Froom Ranch Way	NB	D	Route 4	4.27	E	4.54	E
4	Los Osos Valley	Madonna Rd	Froom Ranch Way	SB	D	Route 5	4.45	E	4.14	D
	Los Osos Valley	Froom Ranch Way	Madonna Rd	NB	D	Route 5	4.38	E	4.35	E
15	Los Osos Valley	Calle Joaquin	US 101 SB Ramps	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Los Osos Valley	US 101 SB Ramps	Calle Joaquin	NB	D		Not Analyzed	N/A	Not Analyzed	N/A
16	Los Osos Valley	US 101 SB Ramps	US 101 NB Ramps	SB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Los Osos Valley	US 101 NB Ramps	US 101 SB Ramps	NB	D		Not Analyzed	N/A	Not Analyzed	N/A
17	Los Osos Valley	US 101 NB Ramps	S. Higuera St	EB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Los Osos Valley	S. Higuera St	US 101 NB Ramps	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
.8	Prado Rd	S. Higuera St	US 101 NB Ramps	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Prado Rd	US 101 NB Ramps	S. Higuera St	EB	D	Route 2	3.83	D	Not Analyzed	N/A
.9	Froom Ranch Way	Dick's Sporting	Los Osos Valley	WB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Froom Ranch Way	Los Osos Valley	Dick's Sporting Goods	EB	D		Not Analyzed	N/A	Not Analyzed	N/A
	Dalidio Dr	Madonna Rd	Froom Ranch Way	SB	D	Route 4	3.64	D	3.65	D
20		Madonna Rd	Froom Ranch Way	SB	D	Route 5	4.20	D	3.96	D
20	Dalidio Dr	iviauoiiiia itu								
	Dalidio Dr Dalidio Dr	Froom Ranch Way	Madonna Rd	NB	D	0.00	Not Analyzed	N/A	Not Analyzed	N/A

Segment 20 transit is southbound for routes 4 and 5

 $Route\ 2\ Serves\ the\ Prado\ Day\ Center\ stop\ during\ the\ AM\ peak\ hour,\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ during\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ the\ PM\ Peak\ Hour\ and\ the\ DMV/Margarita\ stop\ the\ DMV/Margarita\ stop\ the\ the\ DMV/Margarita\ stop\ the\ the\ DMV/Margarita\ stop\ the\ DMV/M$

TABLE 61:
NEAR TERM PLUS PROJECT CONDITIONS SEGMENT LEVEL OF SERVICE: FREEWAY ANALYSIS

				AM Peak Hour			PM Peak Hour		
	Target	Segment	No. of		Density			Density	
Interchange Location	LOS	Type	Lanes	Volume	(pc/mi/ln)	LOS	Volume	(pc/mi/ln)	LOS
US 101 at Los Osos Valley Road									
US 101 NB South of Los Osos Valley Road	С	Freeway	2	3,186	29.1	D	2,538	22.3	С
US 101 SB South of Los Osos Valley Road	С	Freeway	2	1,616	14.2	В	3,516	33.5	D
US 101 at Prado Road				•		•			
US 101 NB South of Prado Road	С	Freeway	2	2,770	24.5	С	2,420	21.2	С
US 101 at Madonna Road				<u>-</u>		-			
US 101 NB South of Madonna Road	С	Freeway	2	2,780	24.6	С	2,806	24.8	С
US 101 SB South of Madonna Road	С	Freeway	2	1,879	16.5	В	3,260	30.0	D

As shown in the Near Term Plus Project Conditions segment analysis tables, most of the pedestrian and transit facilities are projected to operate at unacceptable levels of service as well as several segments for automobile mode and only one segment for the bicycle mode.

Project Proposed Infrastructure Phasing

This section evaluates the proposed phasing of infrastructure included within the project description. Table 7-9 below of the San Luis Ranch Specific Plan is the applicant's proposed infrastructure phasing plan.

Table 7-9 San Luis Ranch Build-Out Phasing Plan

Phase/Trigger	Improvement
Phases 1 and 2 - Residential	Froom Ranch Way Extension
Phases 1 and 2 - Residential	Madonna Road Improvements
Phase 3 - Commercial	Dalidio Drive Improvements
Phase 3 - Commercial	Traffic Signal Improvements
Phase 3 - Commercial	Froom Ranch Way Bridge
After Specific Plan Build-Out	Prado Road Connection - US 101

Froom Ranch Way Extension: Phases 1 & 2

The specific plan describes this infrastructure as the extension of Froom Ranch Way (2-lane collector road with class I path) from the existing Dalidio Drive to the south up to but not including the Froom Ranch Way Bridge. There are no anticipated impacts associated with the proposed timing of these improvements.

Madonna Road Improvements: Phases 1 & 2

The specific plan describes this infrastructure as a right-in and right-out connection with acceleration and deceleration lanes, sidewalk, curb, and gutter, as well as widening to match

the segments east and west of the Specific Plan. There are no anticipated impacts associated with the proposed timing of these improvements.

<u>Dalidio/Prado Improvements: Phase 3</u>

The specific plan describes this infrastructure as widening to two lanes in both directions. Due to level of service and queuing impacts the Prado Road Overcrossing is triggered at part of Phase 2, the projected volumes on Prado Road with that connection occurring under Phase 2 would exceed the 2 lane capacity of Dalidio/Prado Road prior to the improvements proposed to occur at Phase 3 causing a significant impact. Changing timing the Dalidio/Prado Improvement to occur simultaneously with the Prado Road Overcrossing at Phase 2 would mitigate this impact.

Traffic Signal Improvements: Phase 3

The specific plan describes this infrastructure as driveway access controls along Dalidio/Prado for the proposed commercial lots. There are no anticipated impacts associated with the proposed timing of these improvements. However due to level of service and queuing impacts, mitigation for this access point is roundabout control, significant reconfiguration of Dalidio/Prado would be required if construction of the roundabout were to occur at Phase 3.

Froom Ranch Way Bridge: Phase 3

The specific plan describes this infrastructure as completing the Froom Ranch connection to the south providing access to LOVR. If this connection does not occur until Phase 3 all traffic from Phases 1 & 2 as well as left turning traffic from Prado Road to Froom Ranch Way would be forced to use the Dalidio & Madonna Intersection. This volume in combination with the volume already projected at the Dalidio & Madonna Intersection would cause a significant impact to the intersection reducing it to an unacceptable level of service. Changing timing of the Froom Ranch Way Bridge to occur as part of Phases 1&2 would mitigate this impact.

Prado Road Bridge: After Specific Plan Buildout

The specific plan describes this infrastructure as the Prado Road/ Hwy 101 Overcrossing (with or without full ramp access to Hwy 101). Operational assessment under both the existing and near term scenarios identified significant impacts at multiple segments, intersections, and interchanges without the overcrossing and NB ramps in place. Operational assessment under the cumulative scenario identified significant impacts at multiple interchanges without the SB ramps in place. Changing the timing of the Prado Road Overcrossing & NB Ramp to Phase 2 and the SB ramps to Cumulative would mitigate this impact.